

GENERAL NOTES	
1.	THESE PLANS, SPECIFICATIONS, DRAWINGS, NOTES, DETAILS, AND ATTACHMENTS ARE THE SOLE PROPERTY OF COASTLINE ENGINEERING, INC. AND SHALL NOT BE REPRODUCED, OR USED IN CONJUNCTION WITH ANY OTHER PROJECT WITHOUT THE EXPRESS WRITTEN CONSENT OF THIS OFFICE.
2.	ALL ENGINEERING AND CONSTRUCTION, INCLUDING MATERIALS AND WORKMANSHIP, SHALL CONFORM TO THE LATEST REQUIREMENTS OF THE 2016 CALIFORNIA BUILDING CODE WITH THE GOVERNING JURISDICTION AMENDMENTS.
3.	ALL ASTM STANDARDS SHALL BE PER THE LATEST ISSUE OF THE AMERICAN SOCIETY FOR TESTING AND MATERIALS.
4.	GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS, GRADES, ELEVATIONS, AND SITE CONDITIONS PRIOR TO STARTING CONSTRUCTION. THE ARCHITECT AND ENGINEER SHALL BE NOTIFIED IN WRITING OF ANY DISCREPANCIES, INCONSISTENCIES, AND/OR CONDITIONS NEEDING CLARIFICATIONS.
5.	THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. GENERAL CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, WORKERS, AND OTHER PERSONS DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT ARE NOT LIMITED TO, BRACING, SHORING FOR CONSTRUCTION EQUIPMENT, AND SHORING FOR THE STRUCTURE. OBSERVATION VISITS TO THE SITE BY THE ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE ABOVE ITEMS.
6.	IN NO CASE SHALL DIMENSIONS BE SCALED FROM STRUCTURAL PLANS OR DETAILS.
7.	ANY OMISSIONS OR DISCREPANCIES FOUND WITHIN THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER PRIOR TO PROCEEDING.
8.	VIBRATIONAL EFFECTS OF MECHANICAL AND/OR ANY OTHER EQUIPMENT HAVE NOT BEEN CONSIDERED IN THE STRUCTURAL DESIGN.
9.	CONNECTIONS OF ALL ITEMS SUPPORTED BY THE STRUCTURE ARE THE RESPONSIBILITY OF THE DISCIPLINES WHO MAKE THESE ATTACHMENTS.
10.	ALL MATERIALS, SPECIFICATIONS, CONNECTIONS, ETC. SHALL BE AS SHOWN IN THE STRUCTURAL DRAWINGS UNLESS ALTERNATES ARE APPROVED, IN WRITING, BY THE ARCHITECT, ENGINEER, AND OWNER.
11.	WHERE A DETAIL, SECTION, OR NOTE IS SHOWN FOR ONE CONDITION, IT SHALL APPLY FOR ALL LIKE OR SIMILAR CONDITIONS UNLESS NOTED OTHERWISE.
12.	SHOP DRAWINGS FOR ALL FABRICATED MATERIALS SHALL BE SUBMITTED TO AND APPROVED BY THE ENGINEER PRIOR TO FABRICATION.
13.	IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ALL EXISTING UTILITIES, WHETHER INDICATED ON THE CONSTRUCTION DOCUMENTS OR NOT, AND TO PROTECT THEM FROM DAMAGE.
14.	SEE ARCHITECTURAL DRAWINGS FOR ALL WATER PROOFING REQUIREMENTS.
15.	THE CONTRACTOR SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOBSITE CONDITIONS DURING THE COURSE OF CONSTRUCTION, INCLUDING THE SAFETY OF ALL PERSONS OR PROPERTY. THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS. THE CONTRACTOR SHALL DEFEND, INDEMNIFY, AND HOLD THE STRUCTURAL ENGINEER FREE AND HARMLESS FROM ALL CLAIMS, DEMANDS, AND LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPT FOR LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE STRUCTURAL ENGINEER.

CONCRETE NOTES	
1.	CONCRETE MATERIALS, CONSTRUCTION, AND WORKMANSHIP SHALL CONFORM TO ACI 318-11.
2.	THE MINIMUM COMPRESSIVE STRENGTH OF CONCRETE (f_c) AT 28 DAYS SHALL BE AS FOLLOWS (U.N.O.): <ul style="list-style-type: none"> SLAB-ON-GRADE 4,500 PSI FOOTINGS / GRADE BEAMS 4,500 PSI RETAINING WALLS 4,500 PSI CONCRETE OVER METAL DECK 3,000 PSI CAISSONS 4,500 PSI
3.	WHERE CONCRETE COMPRESSION DESIGN STRENGTH IS 3,000 PSI OR GREATER, CYLINDER TESTS ARE REQUIRED.
4.	THE CONCRETE SUPPLIER SHALL BEAR THE RESPONSIBILITY THAT THE MIX DESIGN WILL ATTAIN THE SPECIFIED STRENGTH. ACCEPTANCE OF MIX DESIGN SHALL BE BASED ONLY ON CONFORMANCE OF SPECIFIED COMPRESSION STRENGTH AND SLUMP.
5.	CONCRETE SHALL HAVE A MAXIMUM SLUMP OF 4".
6.	CEMENT SHALL CONFORM TO THE REQUIREMENTS OF ASTM C150, TYPE V. THE WATER/CEMENT RATIO SHALL BE A MAXIMUM OF 0.45.
7.	AGGREGATES SHALL CONFORM TO THE REQUIREMENTS OF ASTM C33 FOR NORMAL WEIGHT CONCRETE AND ASTM C330 FOR LIGHTWEIGHT CONCRETE. THE MAXIMUM SIZE OF THE COARSE AGGREGATE SHALL NOT EXCEED 1/2 SLAB THICKNESS, 3/4", OR THE MINIMUM CLEAR SPACING BETWEEN REINFORCING BARS.
8.	ADMIXTURES SHALL NOT BE USED WITHOUT THE WRITTEN CONSENT OF THE ENGINEER OF RECORD. WHEN SUCH CONSENT IS PROVIDED, ADMIXTURES SHALL BE USED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
9.	READY-MIX CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE WITH THE REQUIREMENTS OF ASTM C94.
10.	MINIMUM CONCRETE COVER FOR REINFORCING STEEL IN NON-PRESTRESSED CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS (U.N.O.): <ul style="list-style-type: none"> CAST AGAINST EARTH 3" EXPOSED TO EARTH OR WEATHER: <ul style="list-style-type: none"> #5 AND LARGER BARS 2" #5 AND SMALLER BARS 1 1/2" NOT EXPOSED TO EARTH OR WEATHER: <ul style="list-style-type: none"> SLABS AND WALLS 3/4" BEAMS AND COLUMNS (TIES, STIRRUPS, SPIRALS) 1 1/2" UNPROTECTED COLUMNS 2"
11.	ALL REINFORCING BARS, WIRE MESH, ANCHOR BOLTS, SLEEVES, AND OTHER CONCRETE INSERTS SHALL BE SECURED IN PLACE AND APPROVED BY THE BUILDING INSPECTOR PRIOR TO PLACING CONCRETE.
12.	ALL CONCRETE SHALL BE CONSOLIDATED WITH MECHANICAL VIBRATORS.
13.	PIPES, DUCTS, SLEEVES, CONDUITS, ETC. SHALL NOT BE PLACED THROUGH CONCRETE UNLESS SHOWN ON THE STRUCTURAL PLANS OR WITH WRITTEN APPROVAL FROM THE ENGINEER OF RECORD.

MASONRY NOTES	
1.	CONCRETE MASONRY UNITS MATERIALS, CONSTRUCTION, AND WORKMANSHIP SHALL CONFORM TO ASTM C90.
2.	CONCRETE MASONRY UNITS SHALL BE MEDIUM WEIGHT WITH $f_m = 1,500$ PSI. ALL UNITS SHALL BE SAMPLED AND TESTED TO VERIFY COMPLIANCE WITH ASTM C95 OR ASTM C90.
3.	MASONRY SHALL BE SOLID GROUTED WITH REINFORCED CELLS.
4.	MORTAR SHALL HAVE A COMPRESSIVE STRENGTH OF 1,900 PSI (MIN.) FOR TYPE 'S' MORTAR AND 2,150 PSI (MIN.) FOR TYPE 'N' MORTAR. MORTAR SHALL CONFORM TO ASTM C270 AND ARTICLES 20.1 AND 2.6A OF TMS 602/ACI 530.1/ASCE 6. TYPE 'S' MORTAR SHALL BE USED FOR ALL WALLS IN CONTACT WITH SOIL AND TYPE 'N' MORTAR FOR ALL OTHER LOCATIONS. ALL HEAD AND BED JOINT THICKNESS SHALL BE BETWEEN 3/8" AND 1/2". BED JOINT THICKNESS OF THE STARTING COURSE OVER THE CONCRETE FOUNDATION MAY BE BETWEEN 1/2" AND 3/4".
5.	GROUT SHALL HAVE A COMPRESSIVE STRENGTH OF 2,000 PSI (MIN.), AS DETERMINED IN ACCORDANCE WITH ASTM C1019. GROUT SHALL CONFORM TO ASTM C476 AND ARTICLE 2.2 OF TMS 602/ACI 530.1/ASCE 6. "FINE" AND "COARSE" AGGREGATE SHALL COMPLY WITH ASTM C404.
6.	THE FIRST COURSE OF BLOCK SHALL BE SET INTO CONCRETE UNLESS A MORTAR KEY IS USED.
7.	HIGH LIFT GROUTING PROCEDURE MAY BE USED FOR HEIGHTS UP TO 24 FT. PER TABLE 7 OF TMS 602. FINE GROUT WITH ADMIXTURES SHALL BE USED FOR LIFTS OVER 12 FT. AND A CLEANOUT SHALL BE PROVIDED AT THE BOTTOM COURSE AT EVERY VERTICAL BAR TO HELP PREVENT VOIDS.

RETAINING WALL NOTES	
1.	ALL BACKFILL MATERIALS SHALL BE GRANULAR, NON COHESIVE SOIL. BACKFILL SHALL BE PLACED IN 12" MAX. HORIZONTAL LIFTS. ALL FILLS SHALL BE COMPACTED TO 90% (MIN.), IF EXPANSIVE SOIL IS ENCOUNTERED, THE ENGINEER OF RECORD SHALL BE NOTIFIED PRIOR TO WALL CONSTRUCTION.
2.	DO NOT PLACE BACKFILL BEHIND WALLS UNTIL THEY HAVE ATTAINED THEIR SPECIFIED DESIGN STRENGTH. WALLS THAT ARE DESIGNED TO BE BRACED BY THE STRUCTURE, SHALL BE SHORED UNTIL THE SUPPORTING MEMBERS ARE IN PLACE.
3.	CONTRACTOR IS RESPONSIBLE FOR ALL DESIGN AND CONSTRUCTION OF ALL UNDERPINNING, CRIBBING, BRACING, AND SHORING REQUIRED FOR THE RETAINING WALL.
4.	WALL DRAINS, 4" Ø MIN., SHALL BE PLACED AT 6 FT. INTERVALS ALONG THE LENGTH OF THE WALL AND LOCATED JUST ABOVE THE LEVEL OF THE SOIL OR PAVING ON THE FRONT FACE OF THE WALL. BACKFILL BEHIND WALL DRAINS OR OPEN HEAD JOINTS MUST BE GRAVEL WITH A MINIMUM THICKNESS OF 12" AND EXTENDING FROM THE TOP OF THE WALL TO THE TOP OF THE FOOTING.

REINFORCING STEEL (REBAR) NOTES	
1.	REINFORCING STEEL SHALL CONFORM TO ASTM A615. ALL REINFORCING SHALL BEAR MILL STOCK IDENTIFICATION.
2.	REINFORCING STEEL GRADE SHALL BE: <ul style="list-style-type: none"> GRADE 60 - #4 BARS AND LARGER GRADE 40 - #3 BARS AND SMALLER ASTM A706 GRADE 60 - ALL WELDED REINFORCING STEEL
3.	REINFORCING STEEL DETAILING, BENDING, AND PLACING SHALL BE IN ACCORDANCE WITH THE LATEST ADDITION OF THE "MANUAL OF STANDARD PRACTICE" BY THE CONCRETE REINFORCING STEEL INSTITUTE.
4.	SEE CONCRETE NOTES FOR CLEAR COVER REQUIREMENTS.
5.	EPOXY COATED REINFORCING STEEL SHALL CONFORM TO ASTM A934.
6.	REINFORCING STEEL SHALL BE CLEAN OF RUST, OIL, GREASE, OR ANY OTHER COATING MATERIAL LIKELY TO IMPAIR BONDING.
7.	SEE TYPICAL REBAR DETAIL ON SHEET S6 FOR LAP SPLICE, BEND, AND HOOK SPECIFICATIONS.
8.	ALL REINFORCEMENT SHALL BE SECURELY TIED IN PLACE BEFORE PLACING CONCRETE OR GROUT.

WOOD NOTES	
1.	ALL LUMBER SHALL CONFORM TO THE GRADES AS SET BY AN INSPECTION AGENCY THAT HAS BEEN APPROVED BY AN ACCREDITATION BODY THAT COMPLIES WITH THE DOC P95 20 OR EQUIVALENT.
2.	FRAMING LUMBER SHALL BE DOUGLAS FIR-LARCH WITH THE FOLLOWING GRADE (U.N.O.): <ul style="list-style-type: none"> STUDS (8'-1" HT. AND LESS) STUD GRADE OR BETTER STUDS (GREATER THAN 8'-1" HT.) #2 OR BETTER SILLS/PLATES AND LEDGERS #2 OR BETTER HEADERS #2 OR BETTER POSTS AND BEAMS #1 OR BETTER
3.	MOISTURE CONTENT OF SAWN LUMBER AT THE TIME OF PLACEMENT SHALL NOT EXCEED 19%.
4.	WOOD BEARING ON CONCRETE OR MASONRY IN CONTACT WITH SOIL SHALL BE PRESSURE TREATED.
5.	WOOD STUDS/POSTS LESS THAN 8" FROM GRADE SHALL BE PRESSURE TREATED.
6.	FASTENERS, INCLUDING NUTS AND WASHERS, IN PRESSURE TREATED WOOD SHALL BE HOT DIPPED ZINC-COATED GALVANIZED STEEL PER ASTM A153. ANCHOR BOLTS MAY HAVE A MECHANICALLY DEPOSITED ZINC COATING WITH WEIGHTS PER ASTM B695, CLASS 55. IT IS ACCEPTABLE TO USE PLAIN CARBON STEEL FASTENERS IN ZINC BORATE TREATED WOOD IN AN INTERIOR, DRY ENVIRONMENT SUCH AS IN A WALL CAVITY.
7.	ALL BOLT HEADS, NUTS, AND LAG SCREWS BEARING ON WOOD SHALL HAVE CUT WASHERS (U.N.O.).
8.	BOLT HOLES IN WOOD SHALL BE DRILLED 1/16" LARGER THAN THE BOLT DIAMETER. BOLT HOLES SHALL BE ACCURATELY ALIGNED AND NOT FORCIBLY DRIVEN.
9.	LEAD HOLES FOR LAG SCREWS IN WOOD SHALL BE BORED AS FOLLOWS: <ul style="list-style-type: none"> FOR SHANK: SAME Ø AND LENGTH AS UNTHREADED END SHANK FOR THREADED PORTION: 75% OF SHANK DIAMETER EQUAL TO THREADED PORTION
10.	GLUED LAMINATED TIMBERS (GLULAM) SHALL BE FABRICATED IN ACCORDANCE WITH THE ANSI A190.1 AND ASTM D3737, USING DOUGLAS FIR INDUSTRIAL GRADE WOOD AND EXTERIOR GLUE WITH INTENDED DRY USE CONDITION. EACH GLULAM SHALL BE GRADE MARKED AND A CERTIFICATE OF CONFORMANCE MUST BE PROVIDED THAT INDICATES CONFORMANCE WITH ANSIAITC A190.1.
11.	SIMPLE SPAN GLULAM BEAMS SHALL BE TYPE 24F-V4 DF/DF AND CANTILEVERED / MULTI-SPAN GLULAM BEAMS SHALL BE TYPE 24F-V4 DF/DF AND CANTILEVERED / MULTI-SPAN GLULAM BEAMS
12.	MANUFACTURED LUMBER PRODUCTS SPECIFIED ON THE DRAWINGS SHALL BE MANUFACTURED BY "WEYERHAEUSER" (ESR-1387, 1153) OR AN ENGINEER APPROVED EQUAL. THE MODULUS OF ELASTICITY FOR PARALLAM (PSL) BEAMS = 2.0E, PARALLAM (PSL) COLUMNS = 1.8E, MICROLAM (LVL) = 2.0E, AND TIMBERSTRAN (LSL) = 1.55E. "BOISE CASCADE" MANUFACTURED LUMBER CAN BE SUBSTITUTED AS SHOWN BELOW: <ul style="list-style-type: none"> TJI 210 = BCI 6000 1.8 PSL (2.0E) = VERSA-LAM 2.0 3100 TJI 230 = BCI 6500 1.8 PSL (1.8E) = VERSA-LAM 2.0 3100 TJI 960 = BCI 60 2.0 LVL (2.0E) = VERSA-LAM 2.0 2900 TJI 560 = BCI 90 2.0 LSL (1.55E) = VERSA-LAM 1.7 2400 / 2650

NAILING SCHEDULE	
CBC TABLE 2304.9.1, FASTENING SCHEDULE	
NAILING OF SAWN WOOD MEMBERS SHALL CONFORM TO THE SCHEDULE BELOW (U.N.O.)	
1.	JOIST TO SILL OR GIRDER, TOENAIL 3-8d
2.	BLOCKING TO JOIST, TOENAIL EACH END 2-8d
3.	1"x6" SUBFLOOR (OR LESS) TO EA. JOIST, FACE NAIL 2-8d
4.	WIDER THAN 1"x6" SUBFLOOR TO EA. JOIST, FACE NAIL 3-8d
5.	2" SUBFLOOR TO JOIST OR GIRDER, BLIND & FACE NAIL 2-16d
6.	SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL 16d @ 16" O.C.
7.	SOLE PLATE TO JOIST OR BLOCKING @ BRACED WALL PANEL 3-16d @ 16" O.C.
8.	TOP PLATE TO STUD, END NAIL 2-16d
9.	STUD TO SOLE PLATE, TOENAIL 4-8d
10.	OR STUD TO SOLE PLATE, END NAIL 2-16d
11.	DOUBLE STUDS, FACE NAIL 16d @ 24" O.C.
12.	DOUBLE TOP PLATES, TYP. FACE NAIL 16d @ 16" O.C.
13.	DOUBLE TOP PLATES, LAP SPLICE 8-16d
14.	BLKG. BETWEEN JOISTS OR RAFTERS TO TOP PLATES, TOENAIL 3-8d
15.	RIM JOIST TO END PLATE, TOENAIL 8d @ 6" O.C.
16.	TOP PLATES, LAPS & INTERSECTIONS, FACE NAIL 2-16d
17.	CONTINUOUS HEADER, TWO PIECES, NAIL ALONG EDGE 16d @ 16" O.C.
18.	CEILING JOIST TO PLATE, TOENAIL 3-8d
19.	CONTINUOUS HEADER TO STUD, TOENAIL 4-8d
20.	CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL 3-16d
21.	CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL 3-16d
22.	RAFTER TO PLATE, TOENAIL 3-8d
23.	1" DIAGONAL BRACE TO EA. STUD & PLATE, FACE NAIL 2-8d
24.	1" x 8" SHEATHING TO EA. BEARING, FACE NAIL 3-8d
25.	WIDER THAN 1" x 8" SHEATHING TO EA. BEARING, FACE NAIL 3-8d
26.	BUILT-UP CORNER STUDS 16d @ 24" O.C.
27.	BUILT-UP GIRDER & BEAMS 20d @ 32" O.C.
28.	2-20d @ ENDS & EA. SPLICE
29.	25. 2" PLANKS TO EA. BEARING 16d
30.	26. COLLAR TIE TO RAFTER, FACE NAIL 3-10d
31.	27. JACK RAFTER TO HIP, TOENAIL 3-10d
32.	28. OR JACK RAFTER TO HIP, FACE NAIL 2-16d
33.	29. ROOF RAFTERS TO 2x RIDGE BEAM, TOENAIL 2-16d
34.	30. OR ROOF RAFTERS TO 2x RIDGE BEAM, FACE NAIL 2-16d
35.	31. JOIST TO BAND JOIST, FACE NAIL 3-16d
36.	32. LEDGER STRIP, FACE NAIL 3-16d
37.	33. WOOD STRUCTURAL PANELS & PARTICLEBOARD SUBFLOOR, ROOF, & WALL SHEATHING (TO FRAMING) 1/2" & LESS, 6d
38.	1/2" - 3/4", 8d
39.	3/4" - 1", 10d
40.	1 1/2" - 1 3/4", 12d
41.	3/4" & LESS, 6d
42.	1/2" - 1", 8d
43.	1 1/4" - 1 1/2", 10d
44.	3/4" & LESS, 6d
45.	1/2", 8d
46.	3/4", 8d
47.	1/2", 6d
48.	3/4", 4d
49.	1/2", 4d
50.	3/4", 6d
51.	1/2", 6d

STEEL NOTES	
1.	STEEL MATERIALS, CONSTRUCTION, AND WORKMANSHIP SHALL CONFORM TO THE 14TH ADDITION OF THE AISC MANUAL OF STEEL CONSTRUCTION AND SHALL BE DETAILED, FABRICATED, AND ERECTED IN CONFORMANCE WITH THE AISC SPECIFICATIONS.
2.	STRUCTURAL STEEL MATERIAL SHALL BE AS FOLLOWS (U.N.O.): <ul style="list-style-type: none"> W SHAPES: ASTM A992, $F_y = 50$ KSI HSS SHAPES (RECTANGULAR): ASTM A500, GRADE B, $F_y = 46$ KSI HSS SHAPES (ROUND): ASTM A500, GRADE B, $F_y = 42$ KSI ALL OTHER SHAPES: ASTM A36, $F_y = 36$ KSI UNHEADED BOLTS & WASHERS: ASTM A36, $F_y = 36$ KSI HEADED BOLTS & THREADED RODS: ASTM A307, GRADE A HIGH STRENGTH BOLTS: ASTM A325M / ASTM A490 (SEE PLANS) SHEAR STUDS: ASTM A108 & A.W.S. D1.1, $F_y = 60$ KSI NUTS: ASTM A563, GRADE A ANCHOR BOLTS & HEAVY HEX HEAD BOLTS INSTALLED IN CONCRETE: ASTM F1554, GRADE 36
3.	STEEL FABRICATORS SHALL FURNISH SHOP DRAWINGS FOR REVIEW BY THE ENGINEER OF RECORD PRIOR TO FABRICATION.
4.	STEEL FABRICATION SHALL BE PERFORMED IN A SHOP THAT IS APPROVED BY THE GOVERNING JURISDICTION.
5.	EXPOSED STEEL SHALL BE PRIMED/PAINTED OR HOT DIPPED GALVANIZED.
6.	Holes shall not be placed in steel members unless specifically detailed on drawings. Holes shall be 1/8" oversized for ordinary connections and 1/4" oversized for anchor bolts (U.N.O.).
7.	GROUTING MATERIAL AT BASE PLATES SHALL BE NON-SHRINK GROUT / DRY PACK WITH A COMPRESSIVE STRENGTH OF $f_c = 6,000$ PSI (MIN.). INSTALL GROUT AFTER COLUMN HAS BEEN PLUMBED AND PRIOR TO FRAMING ERECTION.
8.	HIGH STRENGTH BOLTS SHALL BE PROVIDED WITH HARDENED WASHERS CONFORMING TO ASTM F436.
9.	STRUCTURAL STEEL SHALL BE DELIVERED TO THE JOBSITE FREE OF RUST, MILL SCALE, GREASE, ETC.
10.	STEEL BEAMS WITH SPECIFIED INDUCED CAMBER PER PLAN MAY BE COLD CAMBERED (U.N.O.).

WELDING NOTES	
1.	WELDING SHALL CONFORM TO THE LATEST ADDITION OF THE "STRUCTURAL WELDING CODE" ANSI / AWS D1.1.
2.	WELDS SHALL USE E70 ELECTRODES (EXCEPT REINFORCING STEEL WELDS).
3.	WELDS OF REINFORCING STEEL USING A706 GRADE 60 STEEL SHALL USE E80 ELECTRODES AND SHALL CONFORM TO AWS D1.4 & RG43-77.
4.	SHOP WELDING SHALL BE PERFORMED IN A SHOP THAT IS REGISTERED AND APPROVED BY THE GOVERNING JURISDICTION.
5.	FIELD WELDING SHALL BE CONTINUOUSLY INSPECTED BY A REGISTERED INSPECTOR. ALL FIELD WELDING MUST BE INDICATED ON THE SHOP DRAWINGS.
6.	ALL EXPOSED WELDED CONNECTIONS SHALL BE FILLED AND GROUND SMOOTH AND SUBJECT TO ARCHITECTURAL APPROVAL.
7.	ALL WELDS NOT SPECIFIED SHALL BE CONTINUOUS FILLET WELDS. SIZE OF WELDS SHALL BE BASED ON AISC STANDARDS FOR THICKER MATERIAL CONNECTED.

ABBREVIATIONS			
A.B.	ANCHOR BOLT	GYP. BRD.	GYPSSUM BOARD
ABV.	ABOVE	HDR.	HEADER
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	HGR.	HANGER
ALT.	ALTERNATE	HORIZ.	HORIZONTAL
ARCH.	ARCHITECT	H.S.	HIGH STRENGTH
ASTM	AMERICAN SOCIETY OF TESTING MATERIALS	HSS	HOLLOW STRUCTURAL STEEL
BLK.	BLOCK	HT.	HEIGHT
BLKG.	BLOCKING	IN.	INCHES
BLW.	BELOW	INT.	INTERIOR
BM.	BEAM	JOST	JOIST
B.N.	BOUNDARY NAIL	LBS.	POUNDS
BRG.	BEARING	MAX.	MAXIMUM
BTM.	BOTTOM	M.B.	MACHINE BOLT
BLK.	BLOCK	MECH.	MECHANICAL
CAMB.	CAMBER	MFRG.	MANUFACTURER
C/B	CEILING BEAM	MIN.	MINIMUM
CBC	CALIFORNIA BUILDING CODE	MISC.	MISCELLANEOUS
C.J.	CONTROL JOINT	(N)	NEW
C/J	CEILING JOIST	N/A	NOT APPLICABLE
C.L.	CENTERLINE	NO.	NUMBER
CLR.	CLEARANCE	N.T.S.	NOT TO SCALE
C.M.U.	CONCRETE MASONRY UNIT	O.C.	ON CENTER
COL.	COLUMN	OPNG.	OPENING
CONC.	CONCRETE	PAR.	PARALLEL
CONN.	CONNECTION	PERP.	PERPENDICULAR
CONST.	CONSTRUCTION	PL	PLATE
CONT.	CONTINUOUS	PSF	POUNDS PER SQUARE FOOT
CVR.	COVER	PSI	POUNDS PER SQUARE INCH
d	NAIL PENNY SIZE	PRE.	PRESSURE TREATED
D/B	DECK BEAM	RAD.	RADIUS
DET.	DETAIL	R/B	ROOF BEAM
D.F.	DOUGLAS FIR LARCH	REINF.	REINFORCING
DIA.	DIAMETER	REQ'D.	REQUIRED
D/J	DECK JOIST	RF.	ROOF
(E)	EXISTING	R/R	ROOF RAFTER
EA.	EACH	SCHED.	SCHEDULE
E.F.	EACH FACE	SHTG.	SHEATHING
EMBED.	EMBEDMENT	SIM.	SIMILAR
EN.	EDGE NAIL	SPEC.	SPECIFICATION
E.O.R.	ENGINEER OF RECORD	SQ.	SQUARE
EQ.	EQUAL	STD.	STANDARD
EQUIP.	EQUIPMENT	STL.	STEEL
E.S.	EACH SIDE	S.S.	SELECT STRUCTURAL
EXIST.	EXISTING	STRUCT.	STRUCTURE / STRUCTURAL
EXT.	EXTERIOR	T&B	TOP AND BOTTOM
F/B	FLOOR BEAM	T&G	TONGUE AND GROOVE
F.G.	FINISH GRADE	THK.	THICK
F/J	FLOOR JOIST	TS	TUBE SHAPE
FLR.	FLOOR	THRU	THROUGH
F.N.	FIELD NAIL	T.O.W.	TOP OF WALL
FNDT.	FOUNDATION	U.N.O.	UNLESS NOTED OTHERWISE
FRMG.	FRAMING	VERT.	VERTICAL
FT.	FEET	W.	WIDE FLANGE
FTG.	FOOTING	W/O	WITHOUT
GA.	GAUGE	WD.	WOOD
GALV.	GALVANIZED		
GLB.	GLUED LAMINATED BEAM		
GRD.	GRADE		

DESIGN CRITERIA	
DESIGN LOADS	
ROOF LOADS	FLOOR LOADS
ROOFING: COMPOSITION SHINGLE	FLOORING: HARDWOOD / CARPET
TOTAL DEAD LOAD: 15.0 PSF	TOTAL DEAD LOAD: 15.0 PSF
LIVE LOAD: 20.0 PSF	LIVE LOAD: 40.0 PSF
TOTAL LOAD: 35.0 PSF	TOTAL LOAD: 55.0 PSF
EXTERIOR WALL LOADS	INTERIOR WALL LOADS
MATERIAL: STUCCO	MATERIAL: DRYWALL
TOTAL DEAD LOAD: 16.0 PSF	TOTAL DEAD LOAD: 7.0 PSF
SOIL PROPERTIES	
SOILS REPORT BY:	N/A - VALUES FROM CBC TABLE 1806.2
ALLOWABLE BEARING PRESSURE:	1,500 PSF
PASSIVE PRESSURE:	100 PCF

STATEMENT OF SPECIAL INSPECTION	
DESCRIPTION & TYPE OF INSPECTION REQUIRED	
1.	RETROFIT ANCHOR BOLTS, HOLDOWNS, AND DOWELS USING SIMPSON 'SET-XP' EPOXY INTO CONCRETE (ESR-2508) <ul style="list-style-type: none"> CONTINUOUS SPECIAL INSPECTION TO VERIFY ANCHOR TYPE, ADHESIVE IDENTIFICATION AND EXPIRATION DATE, ANCHOR DIMENSIONS, CONCRETE TYPE, CONCRETE COMPRESSIVE STRENGTH, HOLE DRILLING METHOD, HOLE DIMENSIONS, HOLE CLEANING PROCEDURES, ANCHOR SPACING, EDGE DISTANCES, CONCRETE THICKNESS, ANCHOR EMBEDMENT, TIGHTENING TORQUE, AND ADHERENCE TO THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS.
SPECIAL INSPECTION NOTES:	
A.	SPECIAL INSPECTIONS ARE IN ADDITION TO THOSE REQUIRED BY THE BUILDING DEPARTMENT. SPECIAL INSPECTION IS NOT A SUBSTITUTE FOR INSPECTIONS BY THE CITY INSPECTOR.
B.	THE DUTIES OF THE SPECIAL INSPECTOR SHALL BE IN CONFORMANCE WITH THE REQUIREMENTS OF CHAPTER 17 OF THE CALIFORNIA BUILDING CODE.
C.	THE SPECIAL INSPECTOR MUST BE CERTIFIED BY THE GOVERNING JURISDICTION TO PERFORM THE INSPECTION SPECIFIED, EXCEPT WHERE SPECIFICALLY STATED OTHERWISE.
D.	IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO OBTAIN THE SPECIAL INSPECTOR. FAILURE OF NOTIFICATION FOR INSPECTION MAY RESULT IN COMPLETE REMOVAL AND REPLACEMENT OF ALL WORK SPECIFIED AS NEEDING SPECIAL INSPECTION AT CONTRACTOR'S EXPENSE.
E.	A 'CERTIFICATE OF COMPLIANCE' OF WORK REQUIRING SPECIAL INSPECTION MUST BE COMPLETED AND SUBMITTED TO THE INSPECTION SERVICES DEPARTMENT.
F.	A PROPERTY OWNER'S FINAL REPORT FORM FOR WORK REQUIRED TO HAVE SPECIAL INSPECTIONS, TESTING, AND STRUCTURAL OBSERVATION MUST BE COMPLETED BY THE PROPERTY OWNER, PROPERTY OWNER'S AGENT OF RECORD, ARCHITECT OF RECORD, OR ENGINEER OF RECORD AND SUBMITTED TO THE INSPECTION SERVICES DIVISION.
G.	NOTICE TO THE APPLICANT / OWNER / OWNER'S AGENT / ARCHITECT OR ENGINEER OF RECORD: BY USING THESE PERMITTED CONSTRUCTION DRAWINGS FOR CONSTRUCTION / INSTALLATION OF THE WORK SPECIFIED HEREIN, YOU AGREE TO COMPLY WITH THE REQUIREMENTS OF THE GOVERNING JURISDICTION FOR SPECIAL INSPECTIONS, STRUCTURAL OBSERVATIONS, CONSTRUCTION MATERIAL TESTING, AND OFF-SITE FABRICATION OF BUILDING COMPONENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTIONS, AND AS REQUIRED



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REVISIONS

1/19/17

JOB #: 16-029B
ENGINEER: B.K.
DATE: 3/1/17
SCALE: N.T.S.

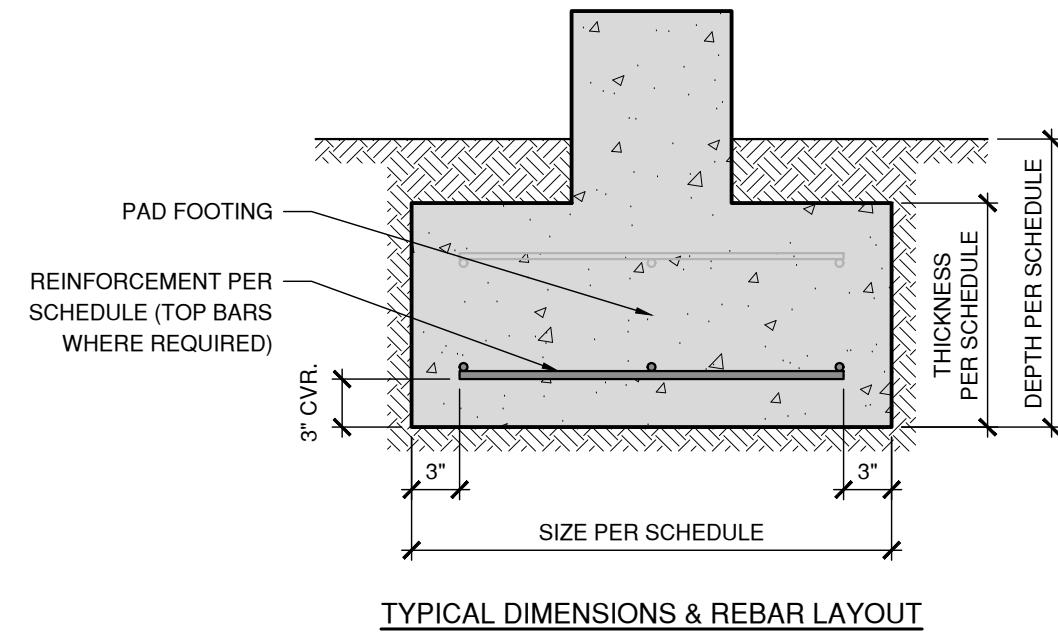
STRUCTURAL SCHEDULES

S3

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SYMBOL	SIZE	DEPTH	THICKNESS (WHERE APPLICABLE)	REINFORCEMENT
A	24" SQUARE	18"	12"	#4 BOTTOM BARS @ 12" O.C. EACH WAY
B	30" SQUARE	18"	12"	#4 BOTTOM BARS @ 12" O.C. EACH WAY
C	36" SQUARE	18"	12"	#4 BOTTOM BARS @ 12" O.C. EACH WAY
D	42" SQUARE	18"	12"	#4 TOP & BOTTOM BARS @ 12" O.C. EACH WAY
E	PER PLAN	18"	12"	#4 TOP & BOTTOM BARS @ 12" O.C. EACH WAY

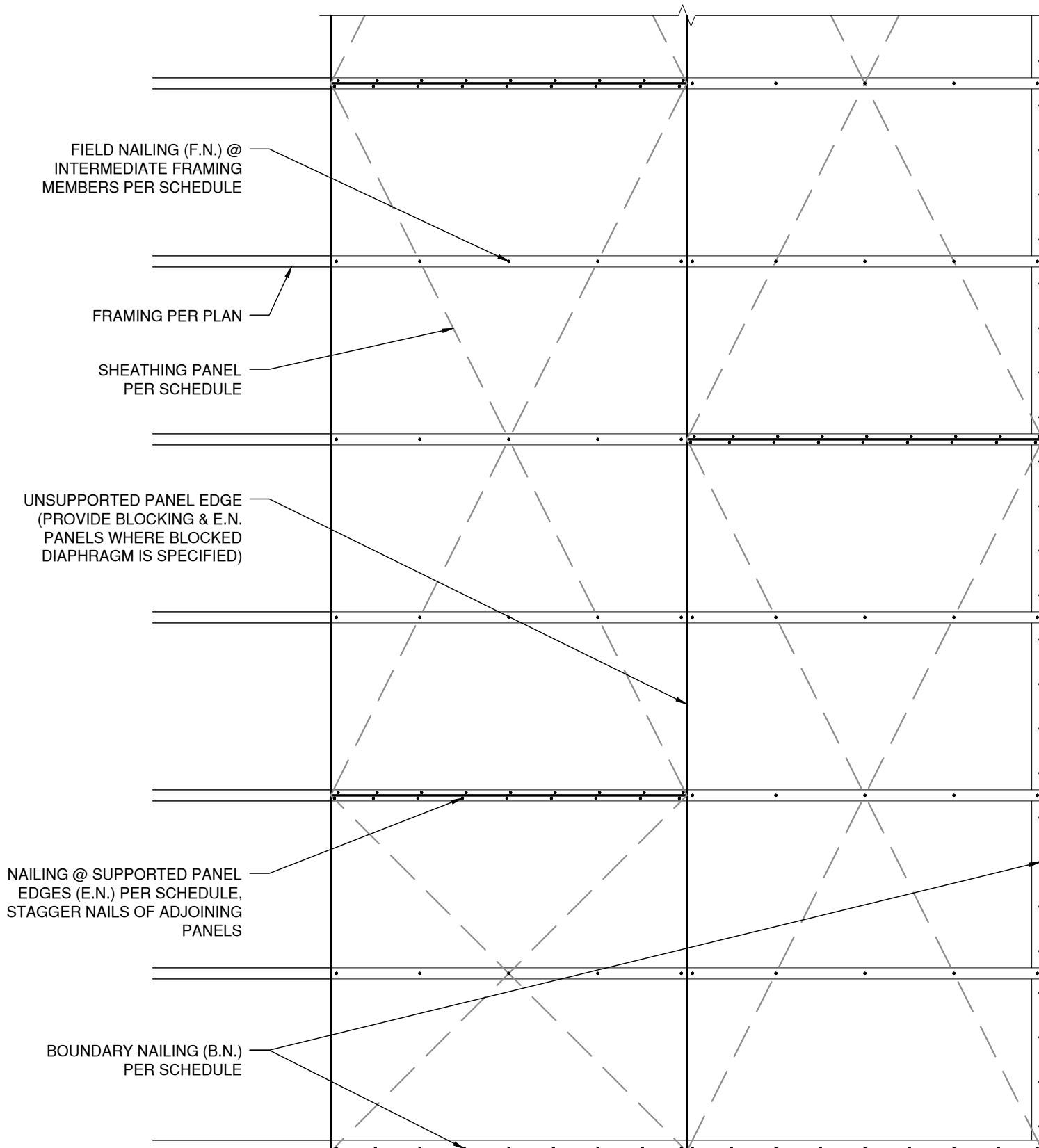
- DEPTH OF FOOTING SHALL BE MEASURED BELOW LOWEST ADJACENT FINAL GRADE. WHERE SPECIFIED, USE DEPTH CALLED OUT ON THE PLAN.
- REINFORCEMENT SHALL MAINTAIN 3" CLEAR DISTANCE FROM SOIL.
- THERE SHALL BE A BAR 3" FROM EACH EDGE OF THE PAD FOOTING WITH SPACING PER SCHEDULE IN BETWEEN.
- CENTER PAD FOOTING ON COLUMN / POST ABOVE (WHERE APPLICABLE).



PAD FOOTING SCHEDULE & TYPICAL DETAILS

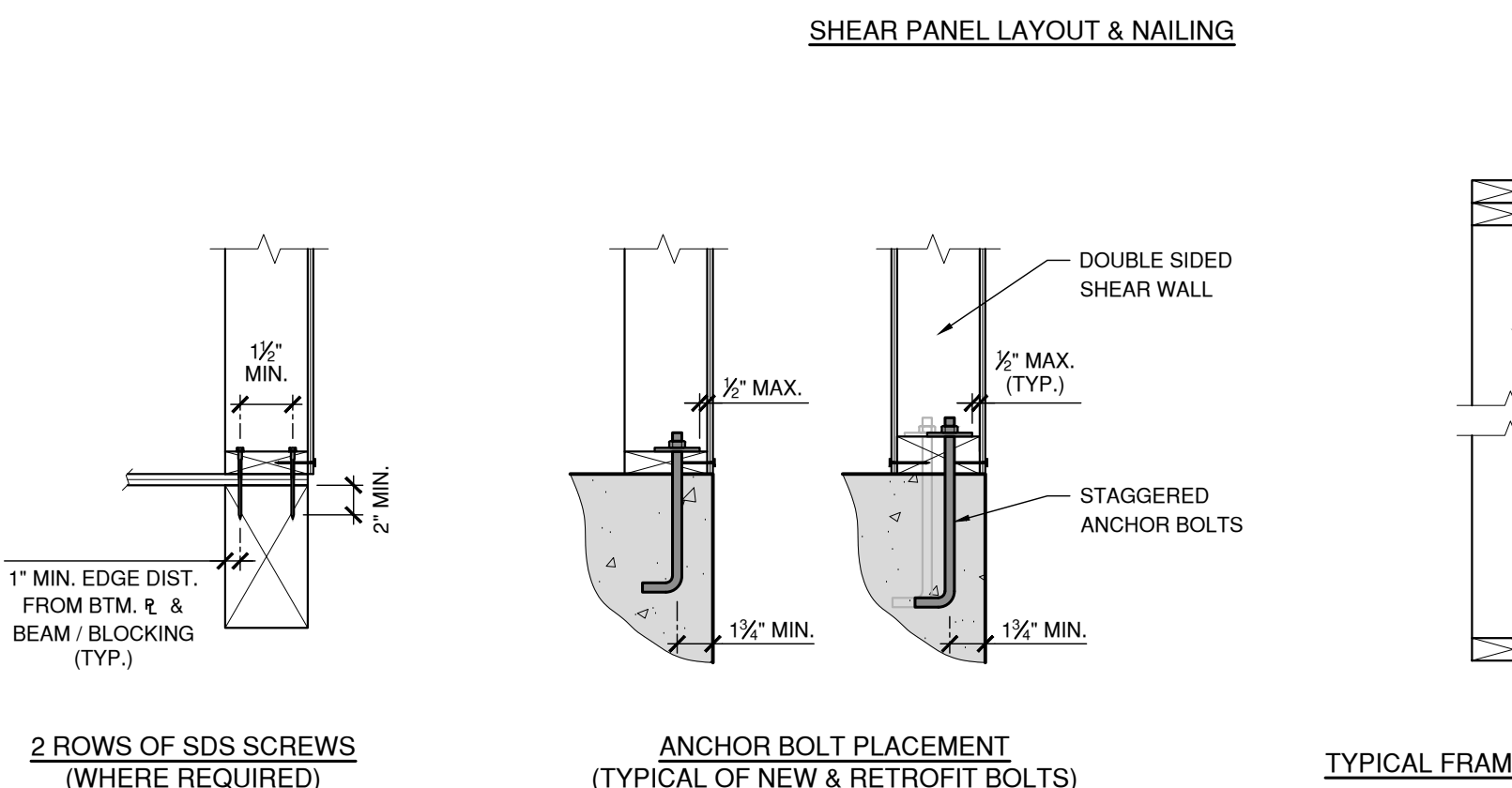
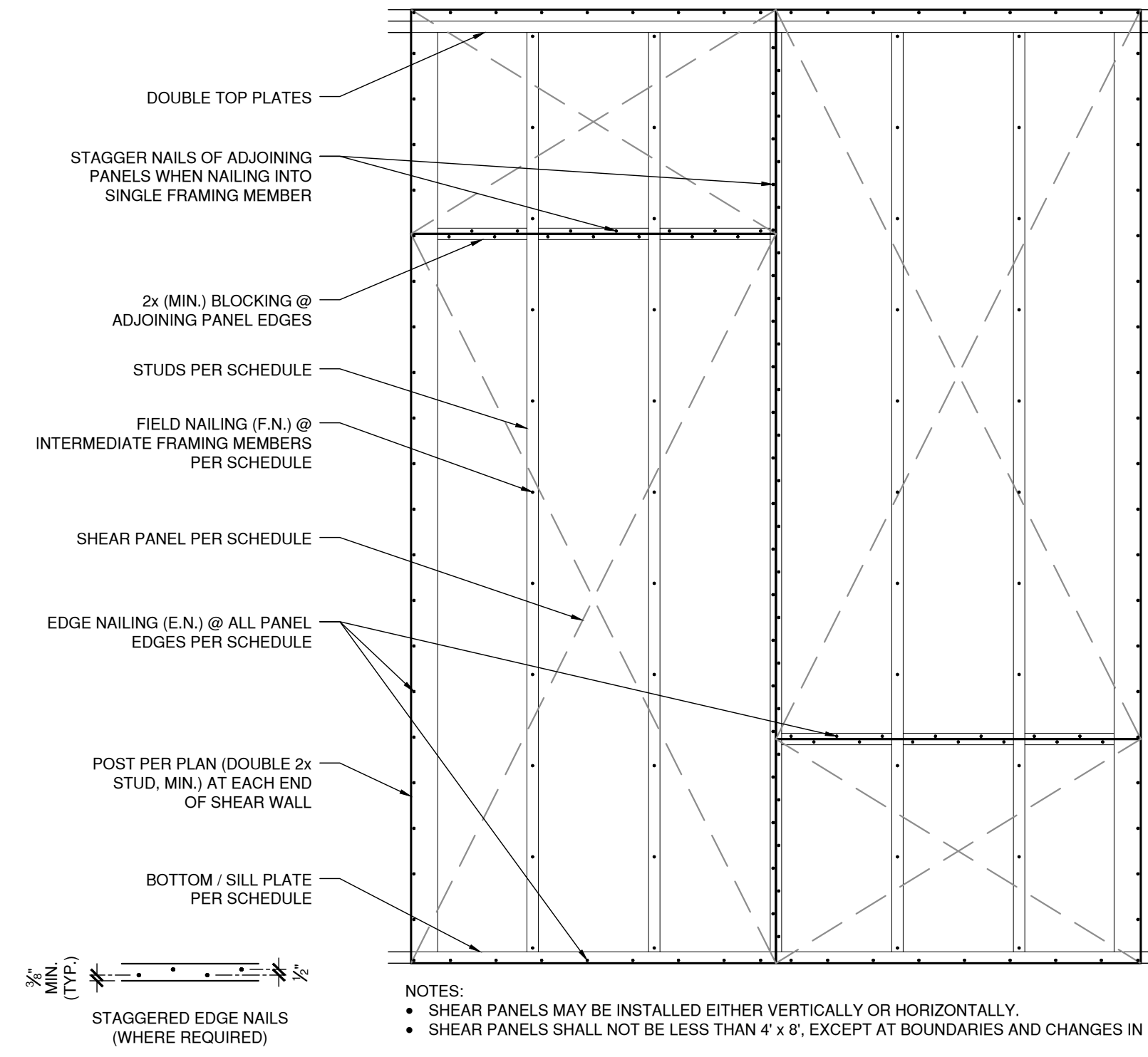
LOCATION	THICKNESS	SPAN RATING	MAX. RAFTER / JOIST SPACING	NAIL TYPE	NAIL SPACING @ DIAPHRAGM BOUNDARIES (B.N.) & SUPPORTED PANEL EDGES (E.N.)
ROOF	1/2"	32/16	32" O.C.	8d	6" O.C.
FLOOR	3/4"	24" O.C.	24" O.C.	10d	6" O.C.

- ALL SHEATHING SHALL BE APA RATED, EXPOSURE 1.
- FLOOR SHEATHING SHALL BE T&G STURD-I-FLOOR AND SHALL BE GLUED AND NAILED.
- PLYWOOD OR OSB CAN BE USED.
- THICKER SHEATHING THAN INDICATED SHALL NOT BE USED WITHOUT WRITTEN CONSENT FROM THE ENGINEER OF RECORD AS NAILS SIZE MAY NEED TO BE ALTERED.
- NAILS AT INTERMEDIATE FRAMING MEMBERS (F.N.) SHALL BE THE SAME SIZE AS INDICATED IN CHART AND BE SPACED @ 12" O.C.
- ONLY COMMON NAILS SHALL BE USED. NAILS SHALL BE DRIVEN WITH THE HEAD OF THE NAIL FLUSH WITH THE SURFACE OF THE SHEATHING.
- NAILS SHALL BE LOCATED AT LEAST 3/8" FROM THE EDGES OF PANELS.



- NOTES:**
- PANELS SHALL BE INSTALLED WITH THE LONG DIMENSION PERPENDICULAR TO FRAMING.
 - PANEL JOINTS PARALLEL TO FRAMING MEMBERS SHALL BE STAGGERED.
 - PANELS SHALL NOT BE LESS THAN 4' x 8' EXCEPT AT BOUNDARIES AND CHANGES IN FRAMING WHERE MINIMUM PANEL DIMENSIONS SHALL BE 24".
 - BOUNDARY NAILING (B.N.) SHALL BE PROVIDED @ ALL BEARING WALLS & FLUSH BEAMS / DRAG MEMBERS.
 - WHERE A BLOCKED DIAPHRAGM IS SPECIFIED ON PLANS, USE 2x4 FLAT BLOCKING AND E.N. PANELS. USE SIMPSON 2" CLIPS TO ATTACH BLOCKING TO FRAMING MEMBERS.

ROOF / FLOOR SHEATHING SCHEDULE & TYPICAL DETAILS



SHEAR WALL SCHEDULE & TYPICAL DETAILS

SYMBOL	CAPACITY (PLF)	SHEAR PANEL & EDGE NAILING (SEE NOTE 1-7)	SILL PLATE & ANCHOR BOLTS (SEE NOTE 12-15-17)	BOTTOM PLATE & NAILS / SCREWS (SEE NOTE 15-14)	SHEAR TRANSFER HARDWARE - RIM / BLKG. TO TOP PLATES (SEE NOTE 16)
1	260 - SEIS. 285 - WIND	3/4" CDX PLYWOOD W/ 8d @ 6" O.C.	2x P.T. PLATE W/ 1/2" O.A.B. (OR 3/8" O.A.B.) @ 36" O.C.	2x PLATE W/ 16d @ 4" O.C. (OR SDS SCREWS @ 12" O.C.)	A35 OR LTP4 @ 24" O.C.
2	350 - SEIS. 350 - WIND	3/4" CDX PLYWOOD W/ 8d @ 4" O.C.	2x P.T. PLATE W/ 1/2" O.A.B. (OR 3/8" O.A.B.) @ 24" O.C.	2x PLATE W/ 16d @ 4" O.C. (OR SDS SCREWS @ 12" O.C.)	A35 OR LTP4 @ 16" O.C.
3	490 - SEIS. 510 - WIND	3/4" CDX PLYWOOD W/ 8d @ 3" O.C. (STAGGERED) (SEE NOTE 9)	2x P.T. PLATE W/ 1/2" O.A.B. @ 20" O.C.	2x PLATE W/ 16d @ 4" O.C. (OR SDS SCREWS @ 8" O.C.)	A35 OR LTP4 @ 12" O.C.
4	640 - SEIS. 640 - WIND	3/4" CDX PLYWOOD W/ 8d @ 2" O.C. (STAGGERED) (SEE NOTE 9)	2x P.T. PLATE W/ 1/2" O.A.B. @ 16" O.C.	2x PLATE W/ 16d @ 3" O.C. (OR SDS SCREWS @ 8" O.C.)	A35 OR LTP4 @ 10" O.C.
5	730 - SEIS. 735 - WIND	3/4" STRUCTURAL 1 PLYWOOD W/ 8d @ 2" O.C. (STAGGERED) (SEE NOTE 9)	2x P.T. PLATE W/ 1/2" O.A.B. @ 14" O.C.	2x PLATE W/ SDS SCREWS @ 5" O.C. (STAGGERED - 2 ROWS @ 10" O.C.) INTO 4x (MIN.) BEAM / BLOCKING	A35 OR LTP4 @ 8" O.C.
6	855 - SEIS. 855 - WIND	3/4" STRUCTURAL 1 PLYWOOD W/ 10d @ 2" O.C. (STAGGERED) (SEE NOTE 9)	2x P.T. PLATE W/ 1/2" O.A.B. @ 12" O.C.	2x PLATE W/ SDS SCREWS @ 4" O.C. (STAGGERED - 2 ROWS @ 8" O.C.) INTO 4x (MIN.) BEAM / BLOCKING	A35 OR LTP4 @ 8" O.C.
7	980 - SEIS. 1025 - WIND	3/4" CDX PLYWOOD EACH SIDE W/ 8d @ 3" O.C. (STAGGERED) (SEE NOTE 9)	3x6 (MIN.) P.T. PLATE W/ 1/2" O.A.B. @ 10" O.C.	3x6 (MIN.) PLATE W/ SDS SCREWS @ 4" O.C. (STAGGERED - 2 ROWS @ 8" O.C.) INTO 4x (MIN.) BEAM / BLOCKING	A35 OR LTP4 @ 6" O.C.
8	1100 - SEIS. 1140 - WIND	3/4" STRUCTURAL 1 PLYWOOD EACH SIDE W/ 8d @ 3" O.C. (STAGGERED) (SEE NOTE 9)	3x6 (MIN.) P.T. PLATE W/ 1/2" O.A.B. @ 10" O.C. (TITEN-HD NOT ACCEPTABLE)	3x6 (MIN.) PLATE W/ SDS SCREWS @ 3" O.C. (STAGGERED - 2 ROWS @ 6" O.C.) INTO 4x (MIN.) BEAM / BLOCKING	A35 OR LTP4 @ 6" O.C.
X	N/A	PREFABRICATED SHEAR WALL (SEE PLANS FOR SHEAR WALL TYPE AND ATTACHMENT SPECIFICATIONS)			

WHEN AN ASTERISK (*) ACCOMPANIES THE SHEAR WALL SYMBOL, THE SHEAR PANEL IS TO BE CONTINUOUS THROUGH ADJACENT WALL FRAMING.

- SHEAR PANELS SHALL BE APPLIED DIRECTLY TO STUD FRAMING.
- ALL EDGES OF ALL SHEAR PANELS SHALL BE SUPPORTED BY AND FASTENED TO FRAMING MEMBERS OR BLOCKING. PANELS MORE THAN ONE VERTICAL PANEL IN HEIGHT SHALL HAVE STAGGERED, SPLICED JOINTS. ALL SPLICED JOINTS SHALL BE BLOCKED WITH 2x BLOCKING (MIN.).
- OSB OF THE SAME TYPE AS SPECIFIED CAN BE USED IN LIEU OF PLYWOOD.
- ONLY COMMON OR GALVANIZED BOX NAILS SHALL BE USED FOR SHEAR PANEL ATTACHMENT. GALVANIZED NAILS SHALL BE HOP DIPPED OR TUMBLED. NAILS SHALL BE DRIVEN WITH THE HEAD OF THE NAIL FLUSH WITH THE SURFACE OF THE SHEATHING.
- ALL SHEAR PANEL NAILS AT INTERMEDIATE FRAMING MEMBERS (F.N.) SHALL BE THE SAME SIZE AS NAILS SPECIFIED FOR EDGE NAILING AND SPACED AT 12" O.C.
- PROVIDE 3/8" MIN. EDGE DISTANCE FOR ALL SHEAR PANEL EDGE NAILING.
- STUDS SHALL BE 2x (MIN.) AND SPACED AT 16" O.C. (MAX.).
- FRAMING AT ADJOINING SHEAR PANEL EDGES SHALL BE 3x (MIN.) AND NAILS SHALL BE STAGGERED. DOUBLE 2x MEMBERS ARE ALLOWED FOR SHEAR WALL TYPES 3 & 4 AND SHALL BE SISTERED W/ 10d @ 4" O.C.
- WHERE SHEAR PANELS ARE APPLIED TO BOTH FACES OF A WALL, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS OR FRAMING MEMBERS SHALL BE 3x (MIN.) AT ADJOINING PANEL EDGES AND NAILS SHALL BE STAGGERED.
- WHERE LTP4 IS INSTALLED OVER SHEATHING (1/2" THICK, MAX.), 8d COMMONS SHALL BE USED.
- ONLY COMMON NAILS SHALL BE USED FOR BOTTOM PLATE ATTACHMENT.
- SEE WOOD NOTES (SHEET S1) FOR ATTACHMENT INTO PRESSURE TREATED LUMBER.
- WHERE BOTTOM PLATE NAILING GOES THROUGH FLOOR SHEATHING THICKER THAN 3/4", USE SDS SCREWS ONLY.
- SDS SCREWS SHALL PENETRATE 2" MINIMUM INTO RIM / BLOCKING. WHERE STAGGERED SDS SCREWS ARE CALLED OUT, THEY SHALL BE IN 2 ROWS WITH 1 1/2" MINIMUM SPACING BETWEEN ROWS AND 1" FROM EACH EDGE OF RIM / BLOCKING.
- ANCHOR BOLTS SHALL BE EMBEDDED 7" (MIN.) INTO NEW POURED IN-PLACE CONCRETE. AT EXISTING CONCRETE, RETROFIT ANCHOR BOLTS SHALL BE 3/4" O THREADED RODS WITH 5" (MIN.) EMBEDMENT INSTALLED USING SIMPSON'S "SET-XP" EPOXY (ESR-2508) WITH SPECIAL INSPECTION. ALTERNATIVELY, RETROFIT ANCHOR BOLTS CAN BE 3/4" x 6" LONG TITEN-HD ANCHORS (ESR-2713) WITH SPECIAL INSPECTION.
- FOUNDATION ANCHOR BOLTS (NEW AND RETROFIT) SHALL HAVE A STEEL PLATE WASHER UNDER EACH NUT NOT LESS THAN 0.229"x3"x3" IN SIZE. THE HOLE IN THE PLATE WASHER IS PERMITTED TO BE DIAGONALLY SLOTTED WITH A WIDTH OF UP TO 3/4" LARGER THAN THE BOLT DIAMETER AND A SLOT LENGTH NOT TO EXCEED 1 1/2", PROVIDED A STANDARD CUT WASHER IS PLACED BETWEEN THE PLATE WASHER AND THE NUT. THE PLATE WASHER SHALL EXTEND TO WITHIN 1/2" OF THE EDGE OF THE SILL PLATE ON THE SIDE(S) WITH THE SHEAR PANEL.
- WELDED STEEL STUDS SHALL BE USED WHERE THE SHEAR WALL STACKS OVER A STEEL BEAM. THE STEEL STUD DIAMETER AND SPACING SHALL MATCH THAT OF THE ANCHOR BOLTS SHOWN IN THE SCHEDULE.

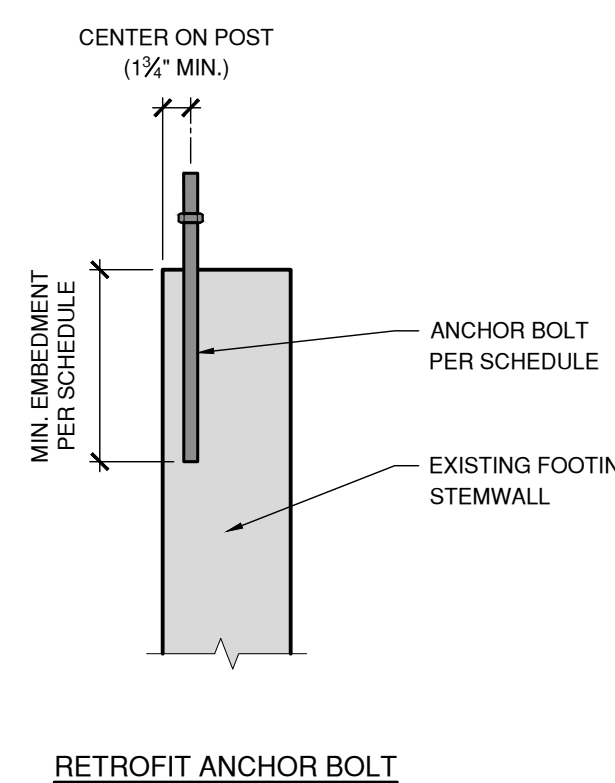
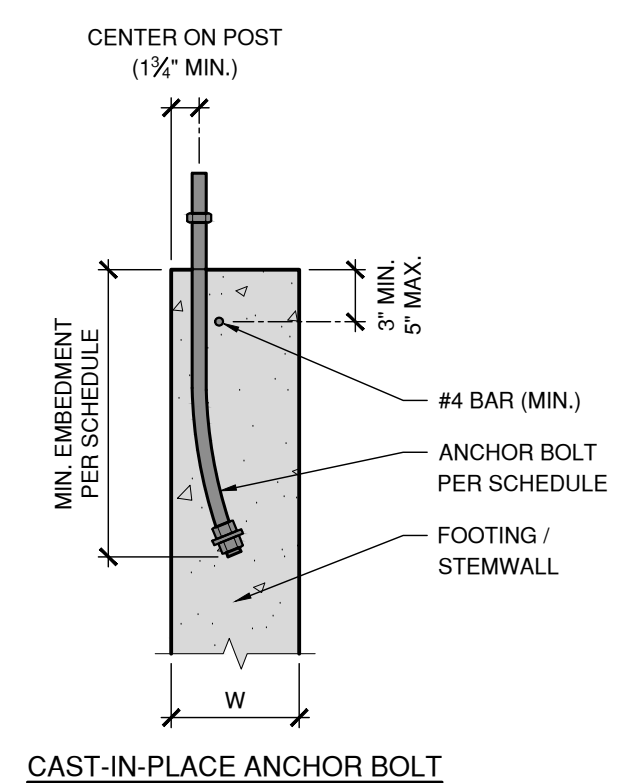
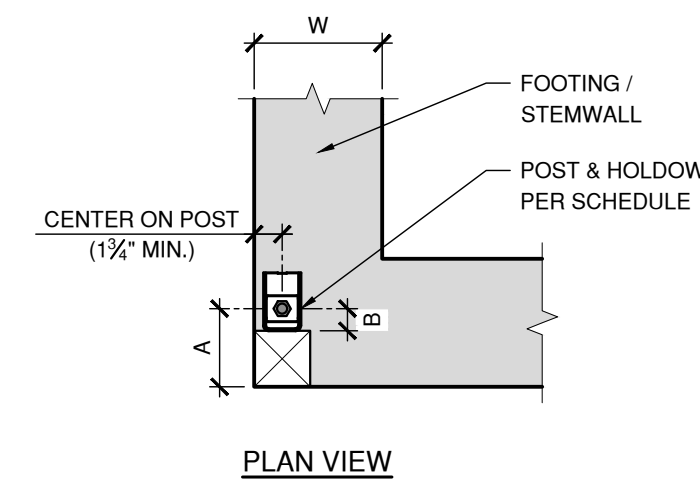
HOLDOWN SCHEDULE NOTES:

- CONCRETE AT ANCHOR BOLT SHALL BE A SINGLE POUR (NO COLD JOINT), UNLESS SPECIFICALLY DETAILED OTHERWISE.
- BOTTOM OF ANCHOR BOLT SHALL HAVE 3" MINIMUM CONCRETE COVER TO SOIL. WHERE FOOTING NEEDS TO BE DEEPENED TO ACCOMMODATE THIS, DEEPENED FOOTING SHALL EXTEND THE LENGTH OF THE SHEAR WALL AND 12" MINIMUM BEYOND THE ANCHOR BOLTS AT EACH END.
- SSTBL SHALL BE SUBSTITUTED FOR SSTB AT 3x SILL PLATES.
- RETROFIT ANCHOR BOLTS INTO CONCRETE SHALL USE SIMPSON'S "SET-XP" EPOXY (ESR-2508) WITH SPECIAL INSPECTION. RETROFIT ANCHOR BOLTS INTO MASONRY SHALL USE SIMPSON'S "SET" EPOXY (ESR-1772) WITH SPECIAL INSPECTION.
- AT RAISED WOOD FLOOR FOUNDATION, USE A "CNW" COUPLER NUT TO ATTACH ANCHOR BOLT TO AN F1554, GRADE 36 THREADED ROD TO EXTEND AND ATTACH TO HOLDOWN.
- DOUBLE 2x POST SHALL BE ATTACHED WITH 10d @ 6" O.C., STAGGERED (U.N.O.).
- WHERE SPECIFIED, USE POST SIZE AS CALLED OUT ON PLANS.
- WHERE EQUAL STRAP LENGTH ON POST AND BEAM IS NOT AVAILABLE DUE TO BEAM DEPTH, STRAP SHALL BE INSTALLED WITH THE BOTTOM OF THE STRAP FLUSH WITH THE BOTTOM OF THE BEAM.

MINIMUM DIMENSIONS

ANCHOR BOLT	A	W
SB3/4x24	4 1/2"	6"
SB3/4x24	4 1/2"	8"
SB1x30	5"	8"
SSTB16	5"	6"
SSTB20	5"	6"
SSTB24	5"	6"
SSTB28	5"	8"

HOLDOWN	B
H DU2	1 1/2"
H DU4	1 1/2"
H DU5	1 1/2"
H DU8	1 1/2"
H DQ8	2 1/2"
H HDQ11	3 1/2"



SYMBOL	HOLDOWN	ANCHOR BOLT (SEE NOTE 1-6)	POST SIZE (SEE NOTE 6-7)			
			2x4 WALL		2x6 (MIN.) WALL	
		TYPE	EMBEDMENT (MIN.)	8'-0" x 10'-0" (MAX.)	10'-0" x 12'-0" (MAX.)	12'-0" x 12'-0" (MAX.)
1	H DU2	OR SB3/4x24 SSTB16	18" 12 1/2"	DBL. 2x	DBL. 2x	DBL. 2x
2	H DU4	OR SB3/4x24 SSTB20	18" 16 1/2"	DBL. 2x	DBL. 2x	DBL. 2x
3	H DU5	OR SB3/4x24 SSTB24	18" 20 1/2"	4x	4x	6x
4	H DU8	OR SB3/4x24 SSTB28	18" 24 1/2"	6x	6x	8x
5	H DQ8	OR SB3/4x24 SSTB28	18" 24 1/2"	6x	8x	8x PSL
6	H HDQ11	SB1x30	24"	8x	6x PSL	8x PSL
7	H DU2	3/4" O THREADED ROD	10"	DBL. 2x	DBL. 2x	DBL. 2x
8	H DU4	3/4" O THREADED ROD	12"	DBL. 2x	DBL. 2x	4x6
9	H DU5	3/4" O THREADED ROD	15"	6x	6x	6x

SYMBOL	HOLDOWN	POST SIZE (SEE NOTE 6-7)				FRAMING BELOW (SIZE PER PLAN)	NOTES
		2x4 WALL		2x6 (MIN.) WALL			
		8'-0" x 10'-0" (MAX.)	10'-0" x 12'-0" (MAX.)	12'-0" x 12'-0" (MAX.)	12'-0" x 12'-0" (MAX.)		
10	MSTC40	DBL. 2x	DBL. 2x	DBL. 2x	DBL. 2x	POST	• STRAP LENGTH SHALL BE EQUAL ON EACH POST
11	MSTC52	DBL. 2x	DBL. 2x	4x	DBL. 2x	POST	• STRAP LENGTH SHALL BE EQUAL ON EACH POST
12	MSTC66	4x	4x	6x	DBL. 2x	POST	• STRAP LENGTH SHALL BE EQUAL ON EACH POST
13	CMST14	4x	6x	8x	4x	POST	• EACH POST SHALL HAVE (2) 16d OR (3) 10d
14	MSTC28	DBL. 2x	DBL. 2x	DBL. 2x	DBL. 2x	FLUSH BEAM	• STRAP LENGTH SHALL BE EQUAL ON POST & BEAM WHERE EQUAL LENGTH IS NOT AVAILABLE DUE TO BEAM DEPTH. (SEE NOTE 8)
15	MSTC66B3	DBL. 2x	DBL. 2x	4x	DBL. 2x	FLUSH BEAM	• FOR 10" DEEP BEAM, USE MSTC48B3
16	(2) ST6224	6x	6x	6x	6x	FLUSH BEAM	• STRAP LENGTH SHALL BE EQUAL ON POST & BEAM WHERE EQUAL LENGTH IS NOT AVAILABLE DUE TO BEAM DEPTH. (SEE NOTE 8)
17	HST3	6x	6x	8x	6x	FLUSH BEAM	• BEAM MUST BE PSL (OR EQUIV.) • STRAP LENGTH SHALL BE EQUAL ON POST & BEAM WHERE EQUAL LENGTH IS NOT AVAILABLE DUE TO BEAM DEPTH. (SEE NOTE 8)
18	MSTC40	DBL. 2x	DBL. 2x	DBL. 2x	DBL. 2x	HEADER / DROPPED BEAM	• STRAP LENGTH SHALL BE EQUAL ON POST & BEAM WHERE EQUAL LENGTH IS NOT AVAILABLE DUE TO BEAM DEPTH. (SEE NOTE 8)
19	MSTC66B3	DBL. 2x	DBL. 2x	4x	DBL. 2x	HEADER / DROPPED BEAM	• STRAP LENGTH SHALL BE EQUAL ON POST & BEAM WHERE EQUAL LENGTH IS NOT AVAILABLE DUE TO BEAM DEPTH. (SEE NOTE 8)

HOLDOWN SCHEDULE & TYPICAL DETAILS

ROOF / FLOOR SHEATHING SCHEDULE & TYPICAL DETAILS



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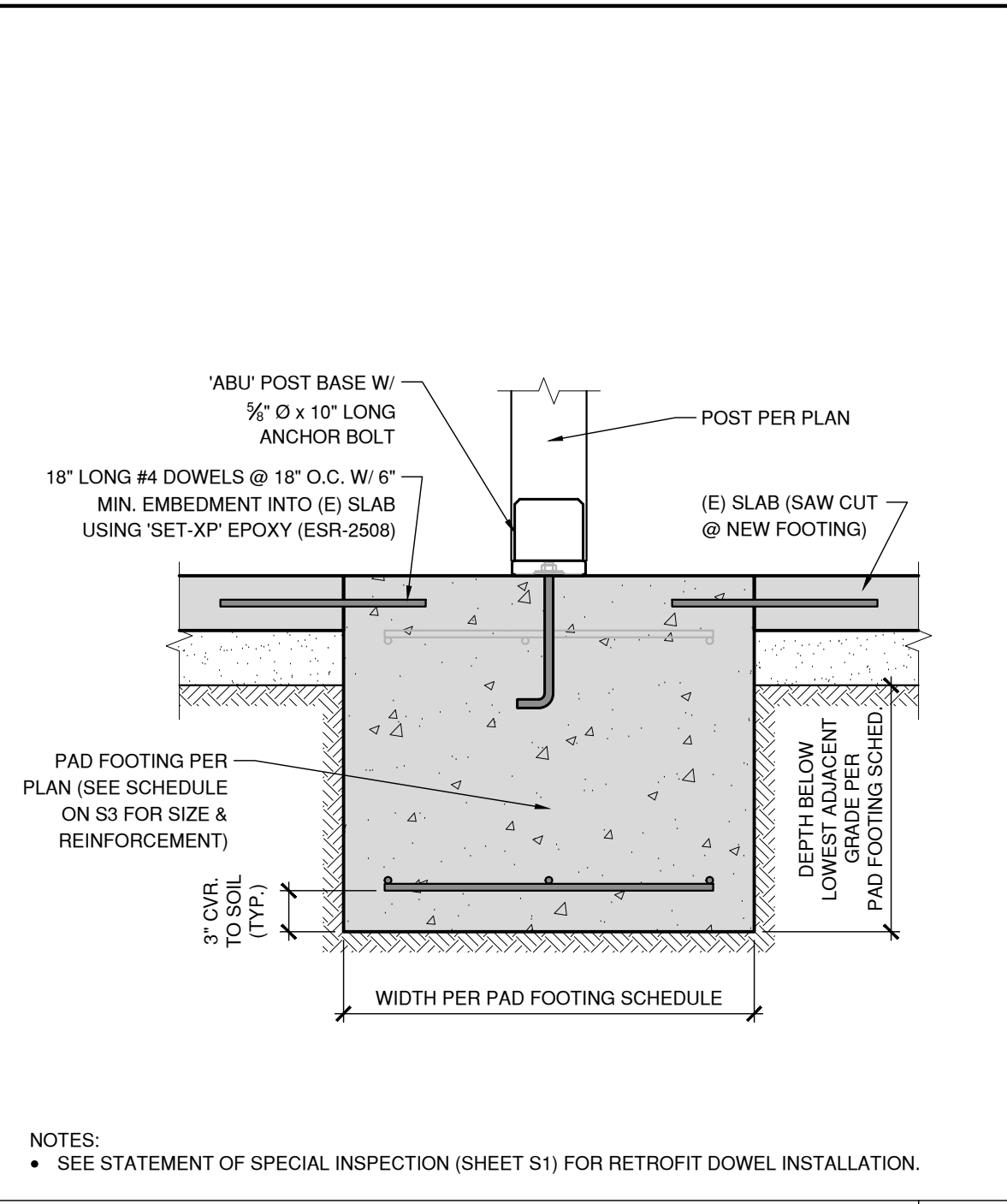
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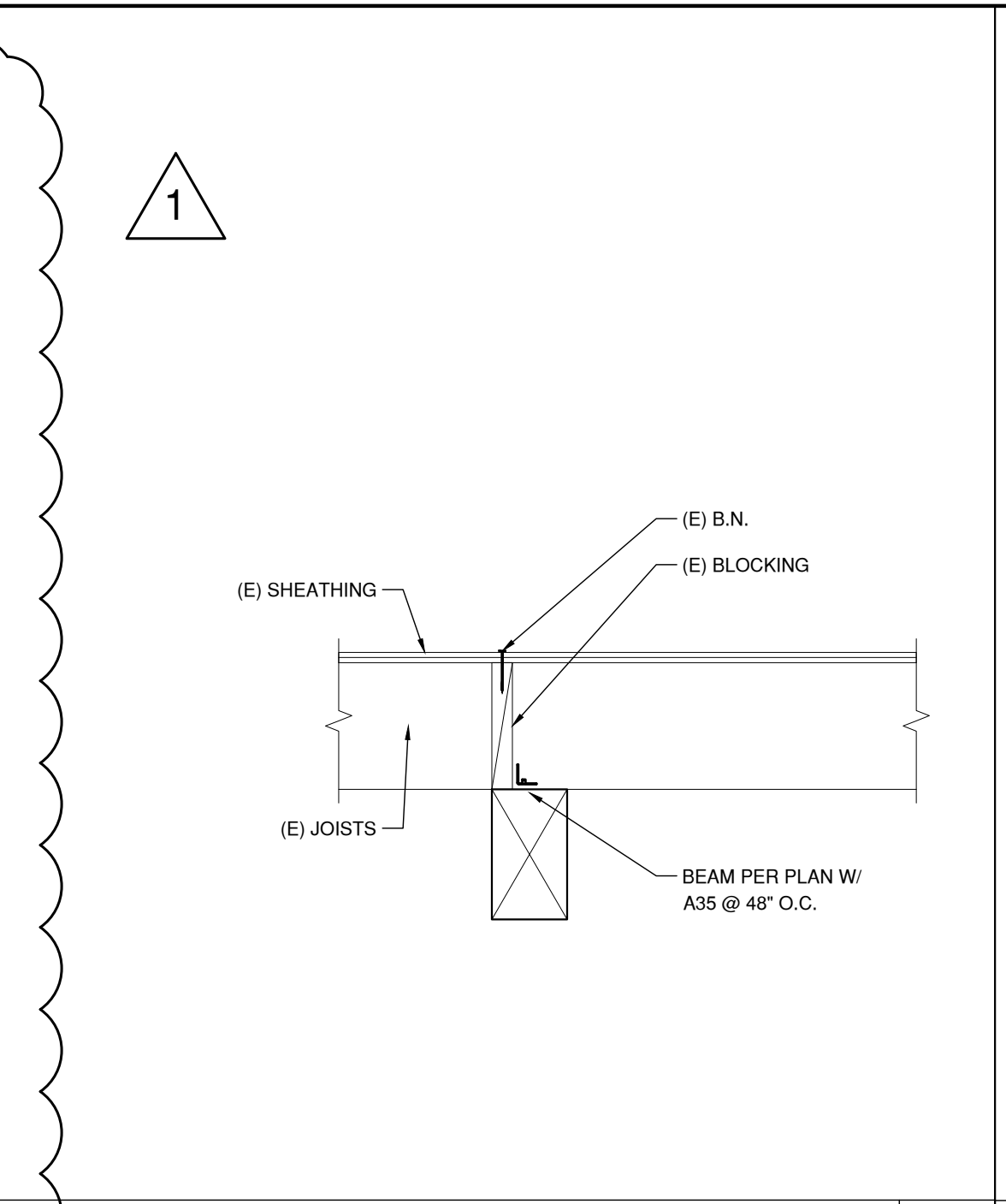
STRUCTURAL DETAILS

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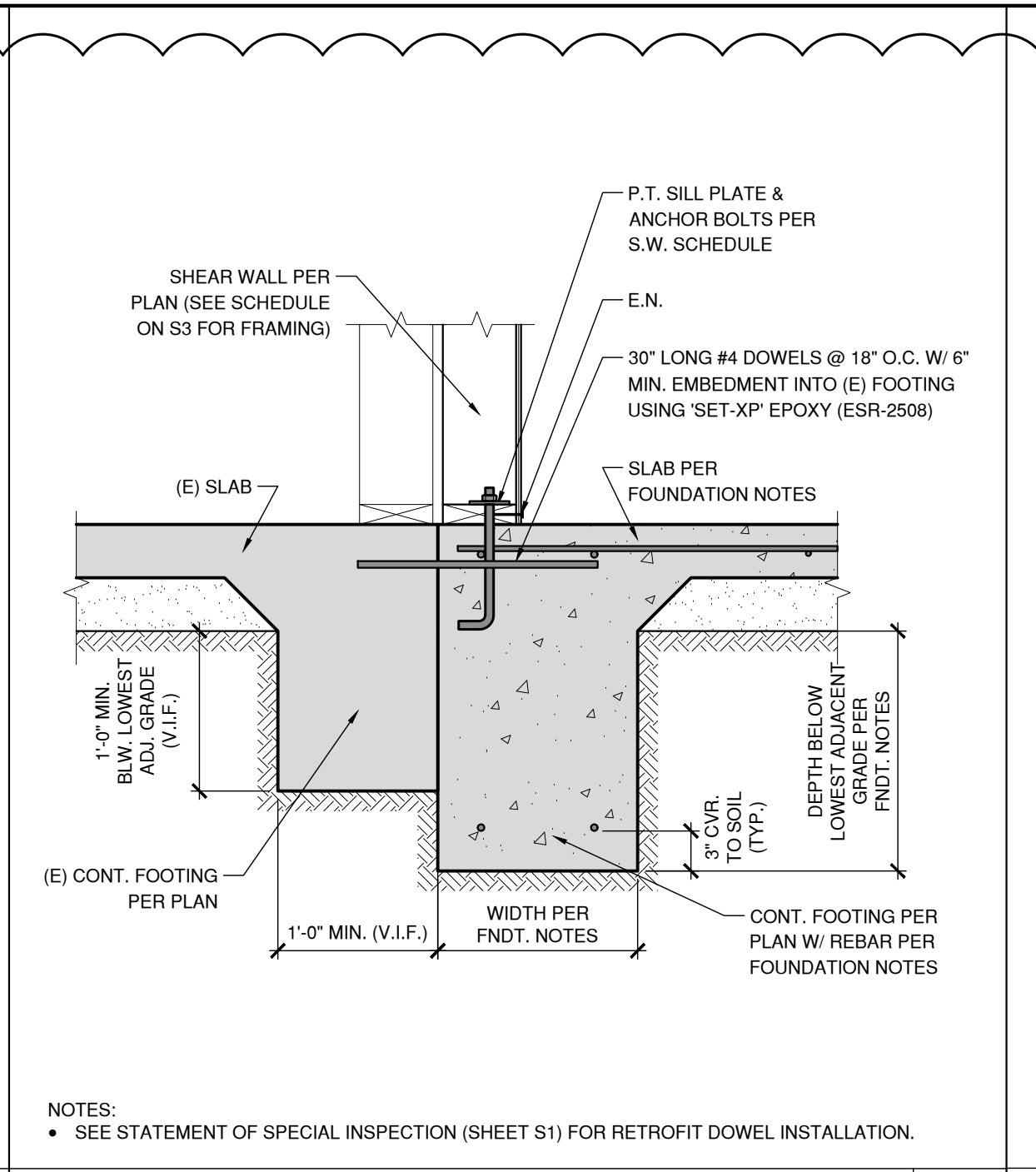
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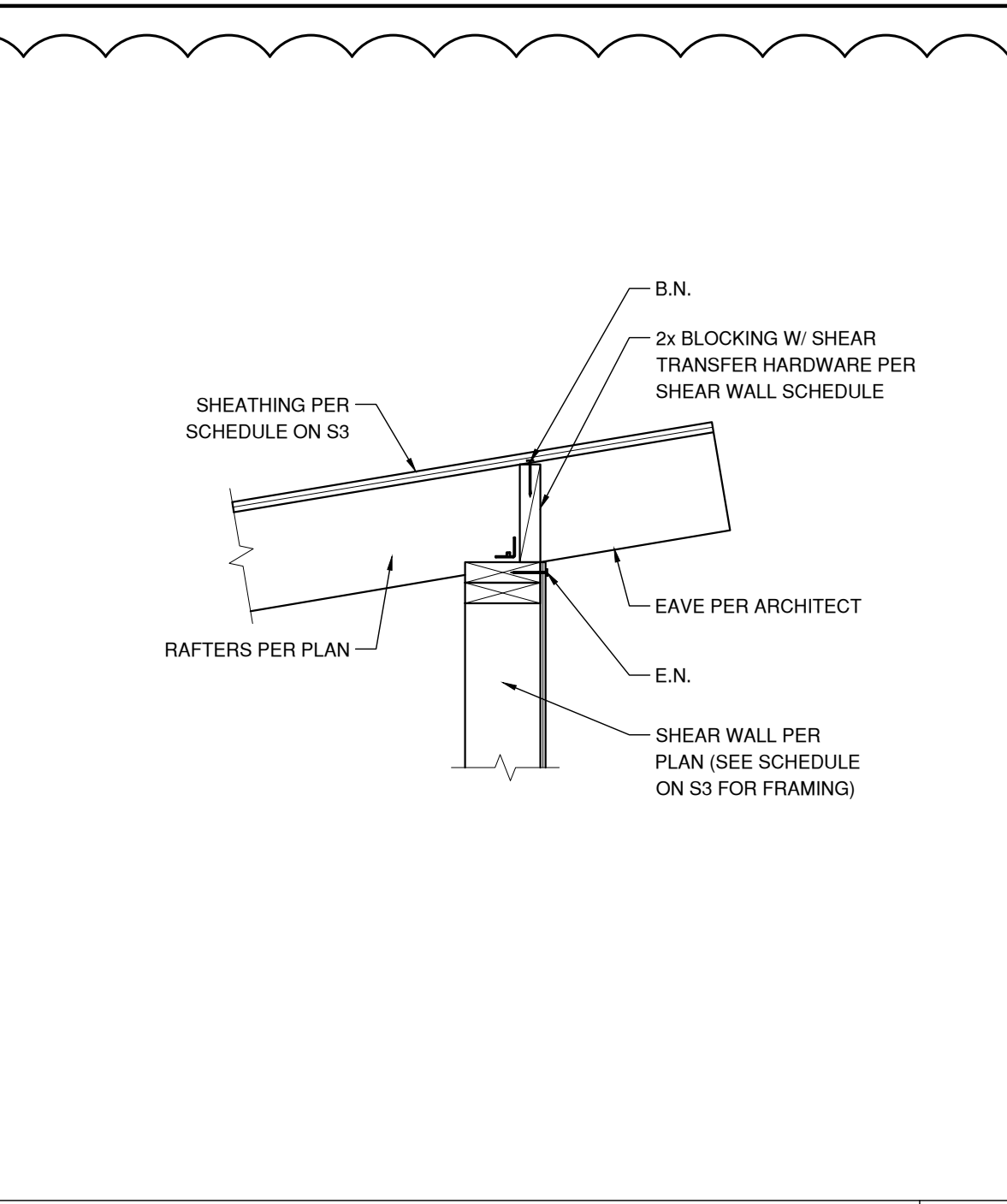
NOTES:
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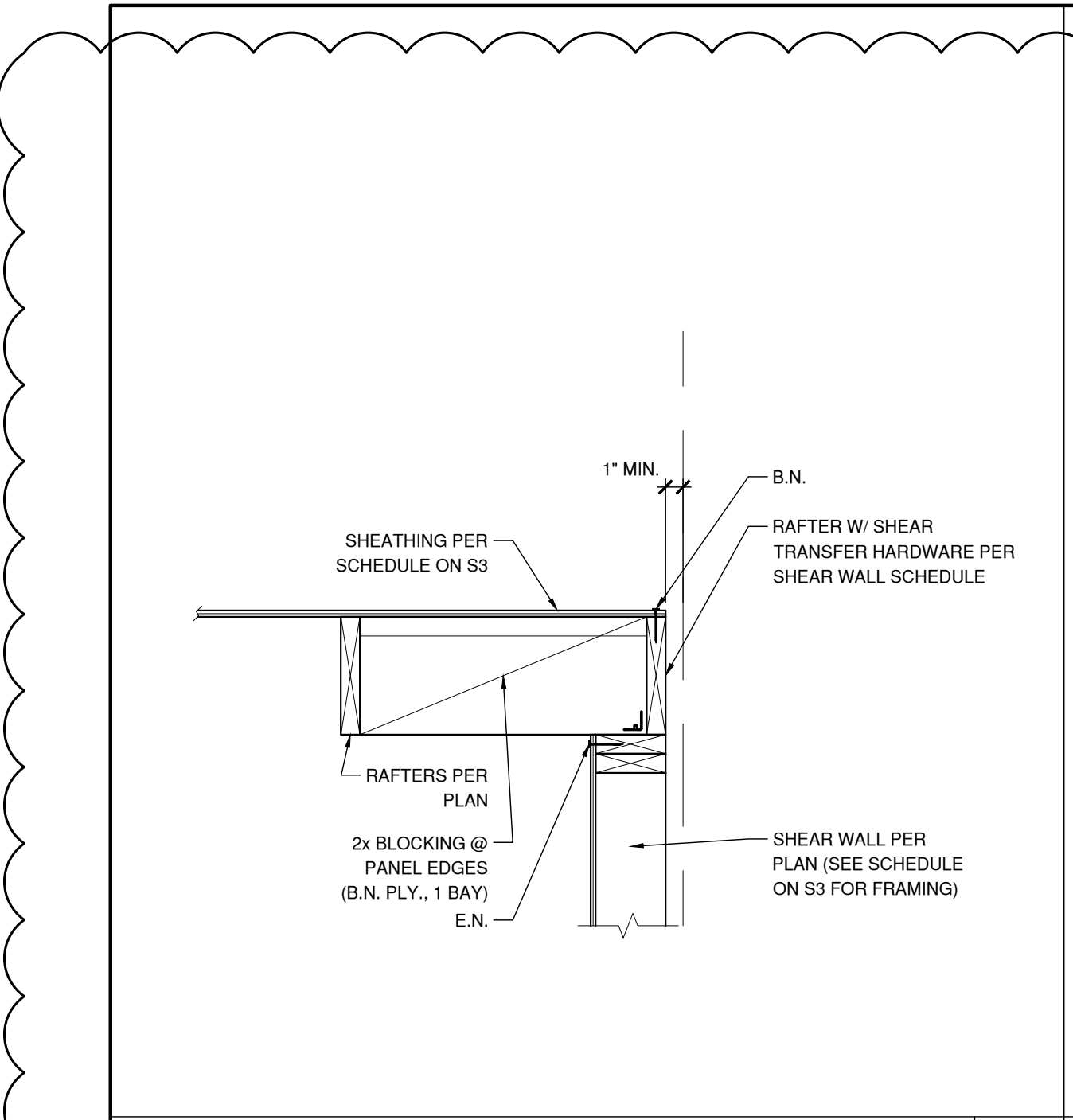
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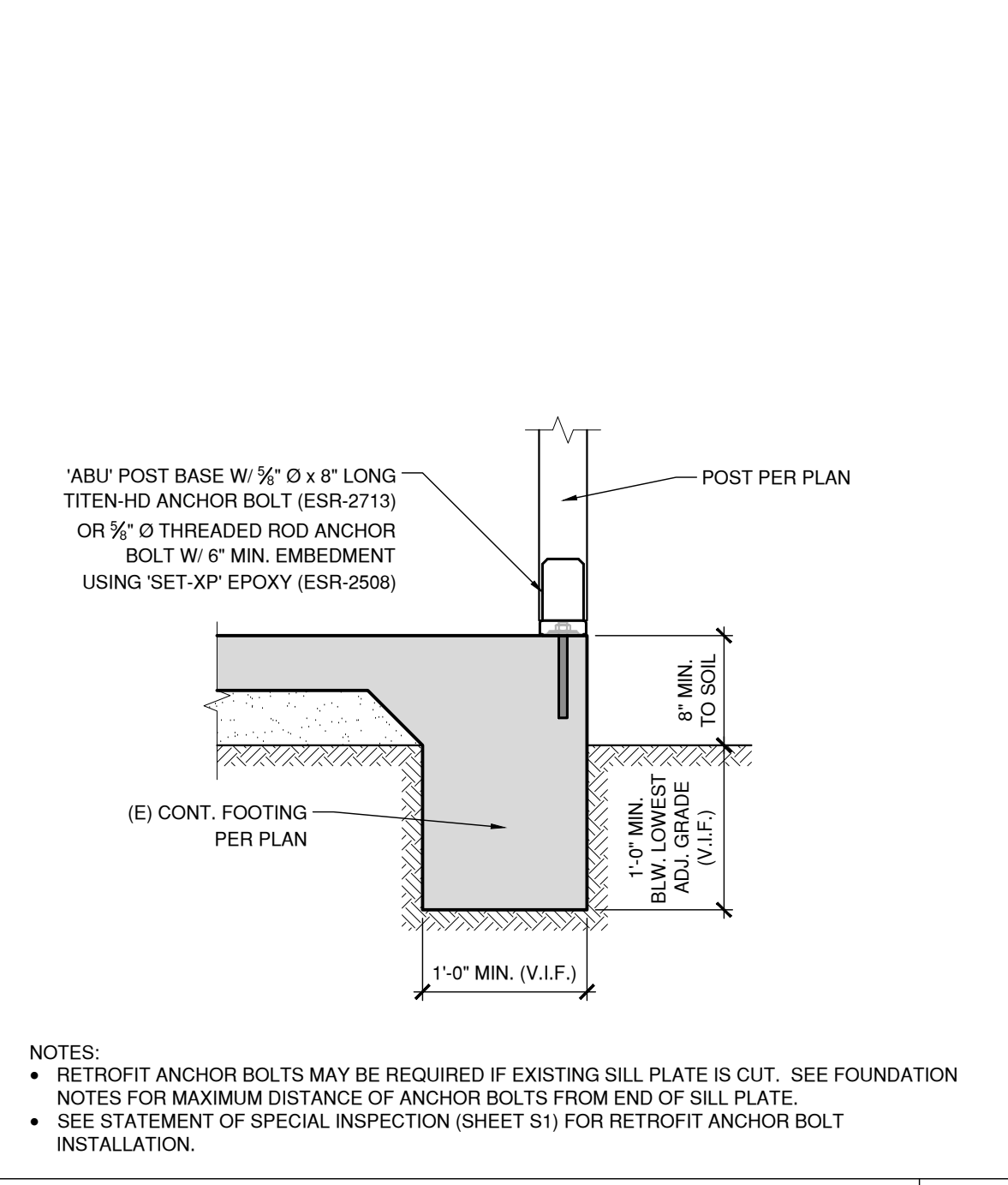
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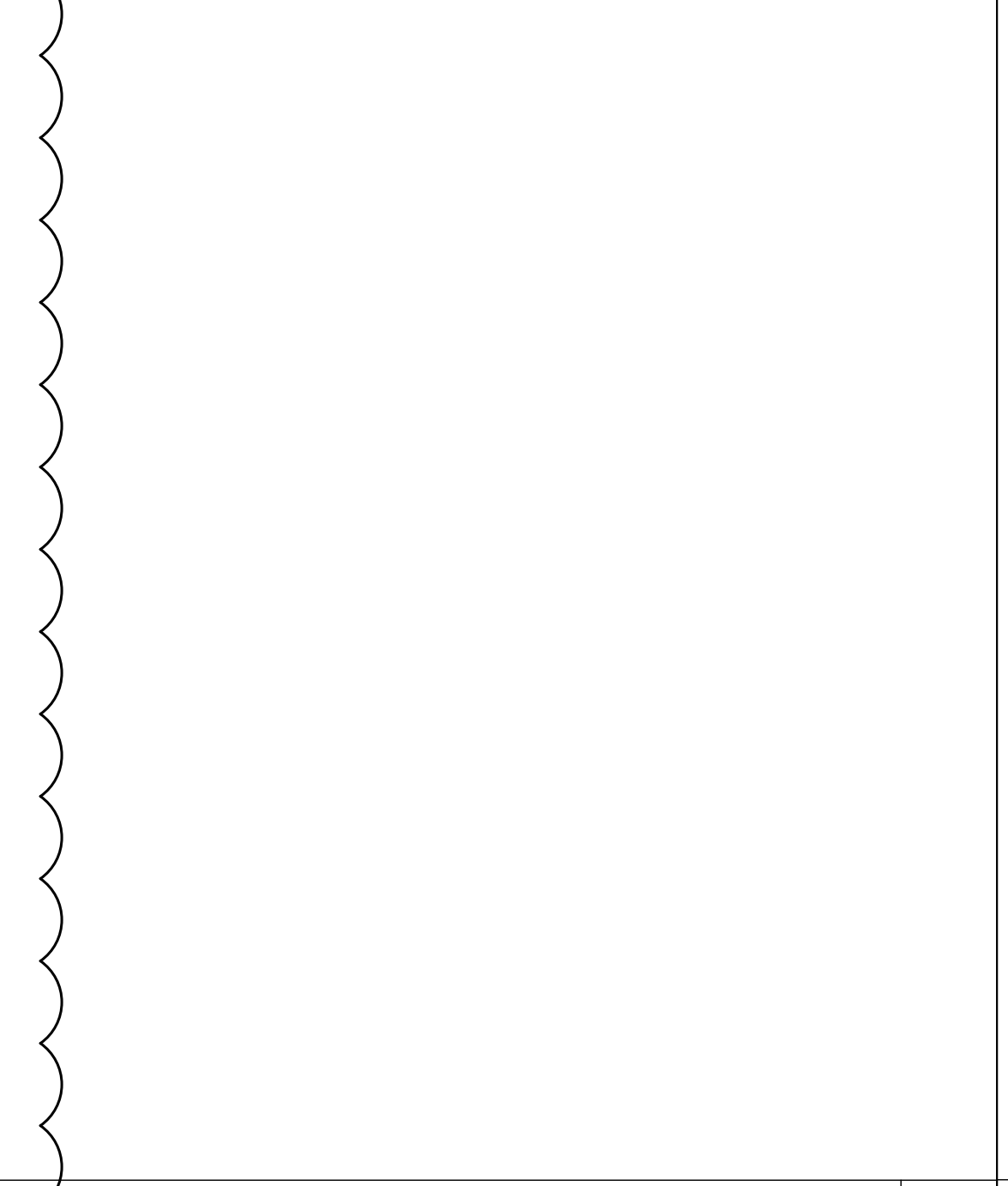
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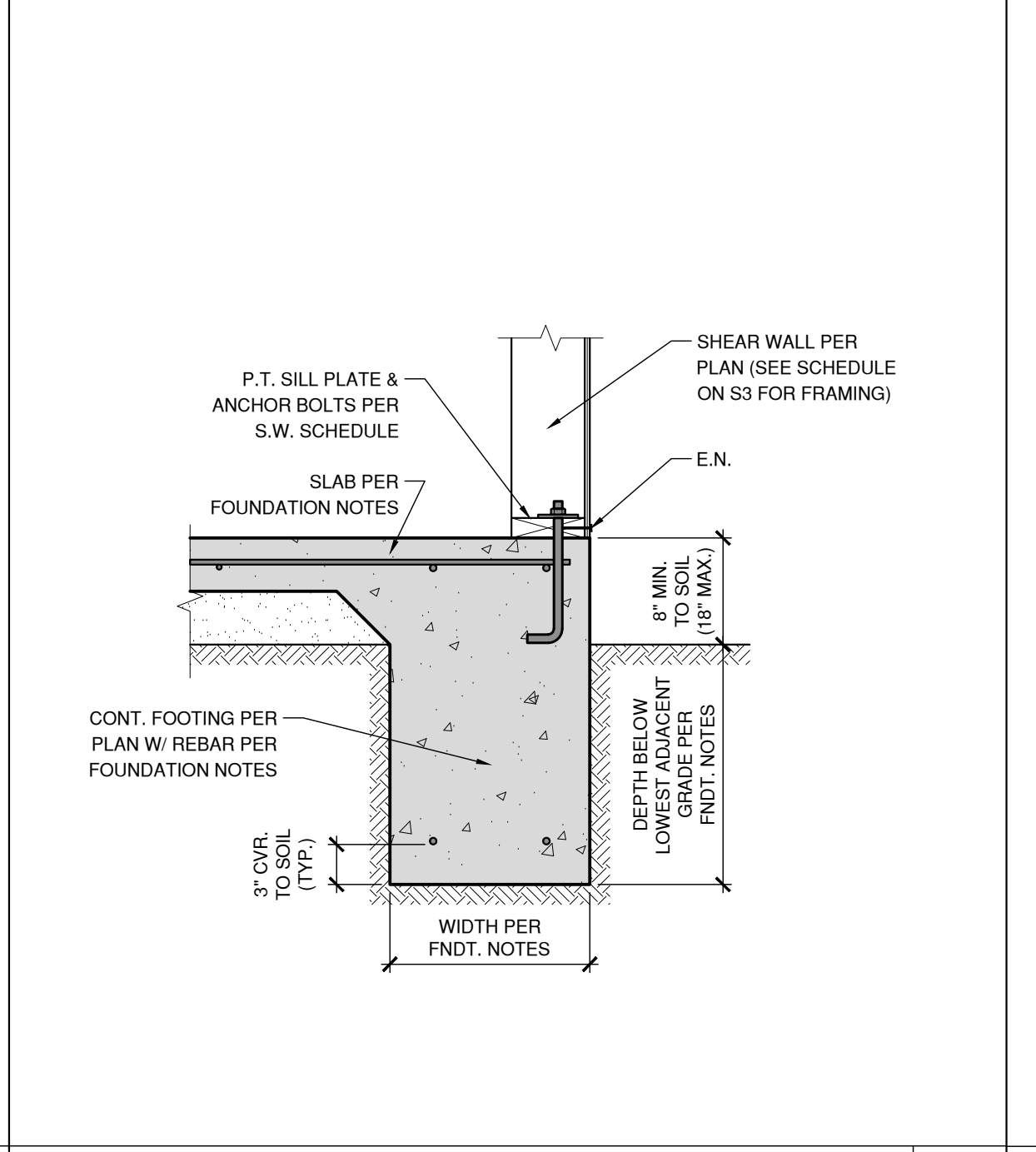
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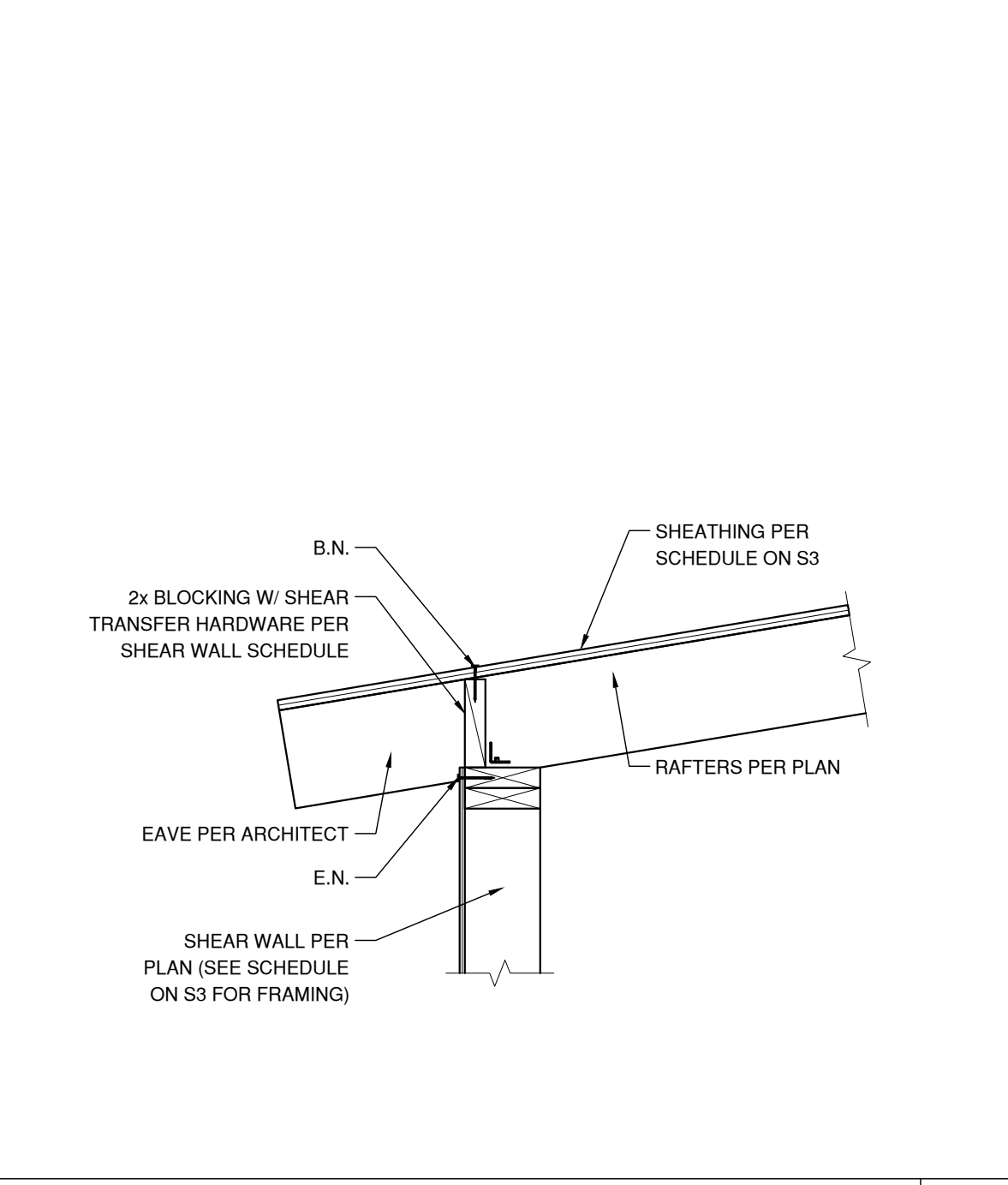
NOTES:
• RETROFIT ANCHOR BOLTS MAY BE REQUIRED IF EXISTING SILL PLATE IS CUT. SEE FOUNDATION NOTES FOR MAXIMUM DISTANCE OF ANCHOR BOLTS FROM END OF SILL PLATE.
• SEE STATEMENT OF SPECIAL INSPECTION (SHEET S1) FOR RETROFIT ANCHOR BOLT INSTALLATION.



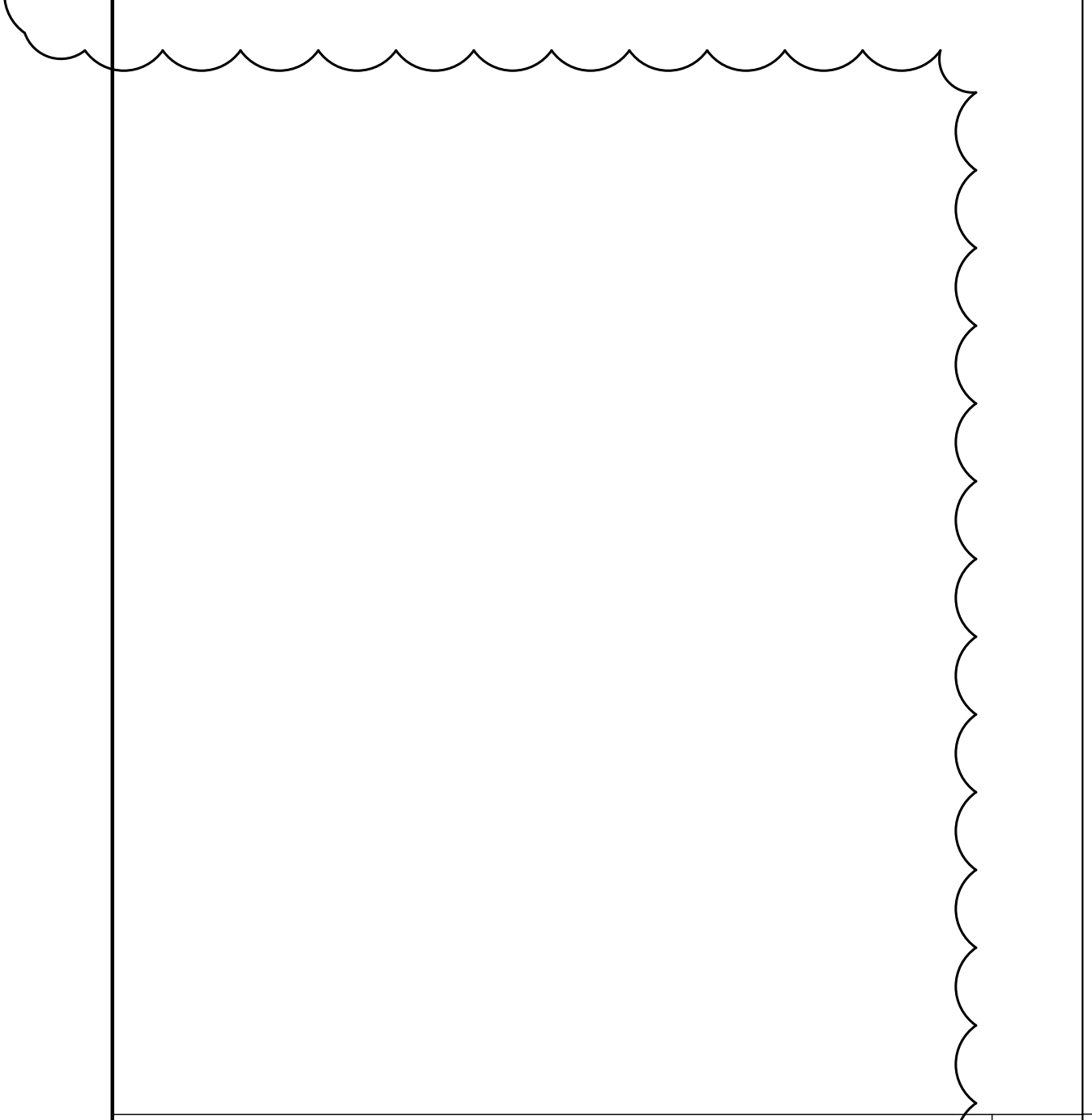
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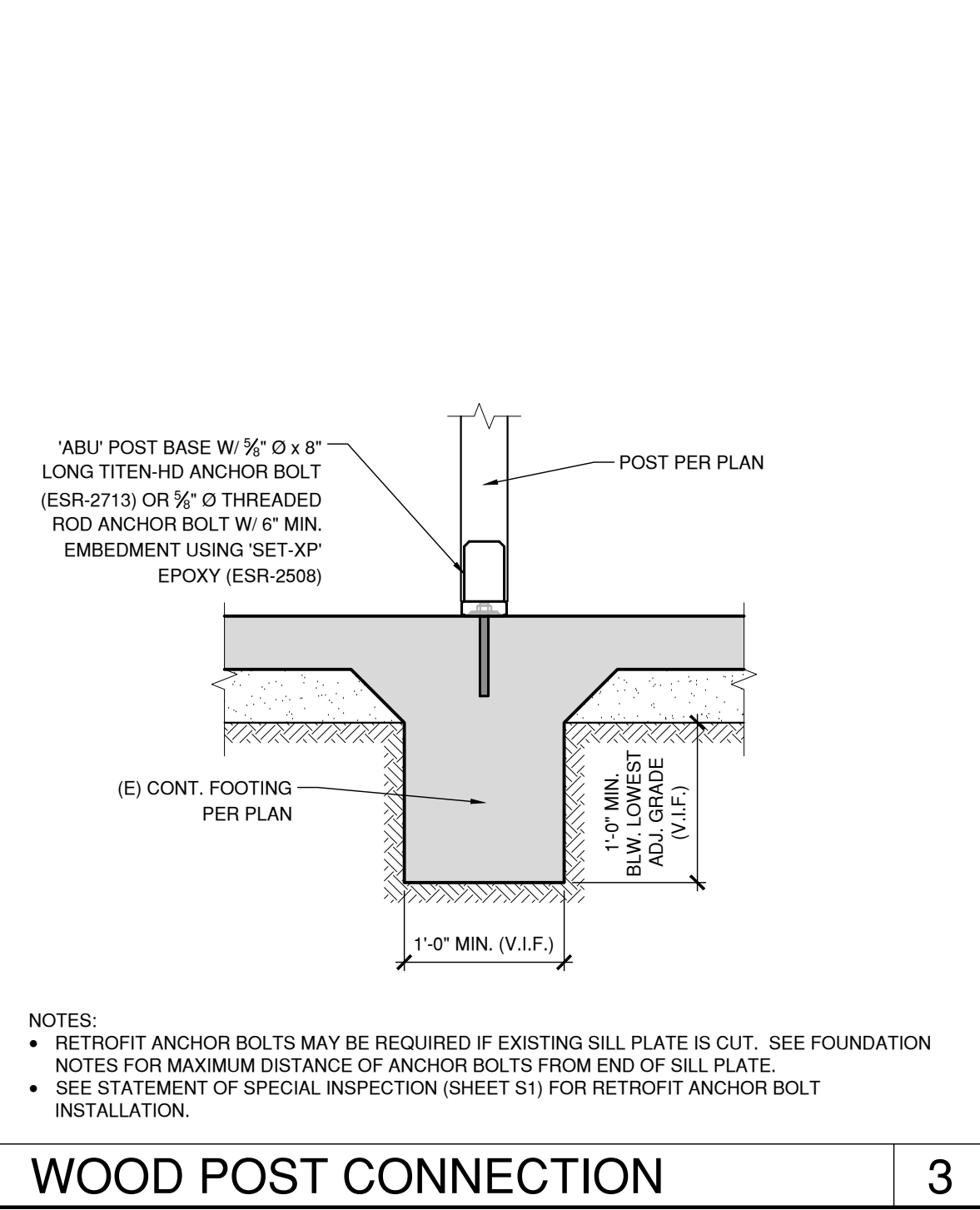
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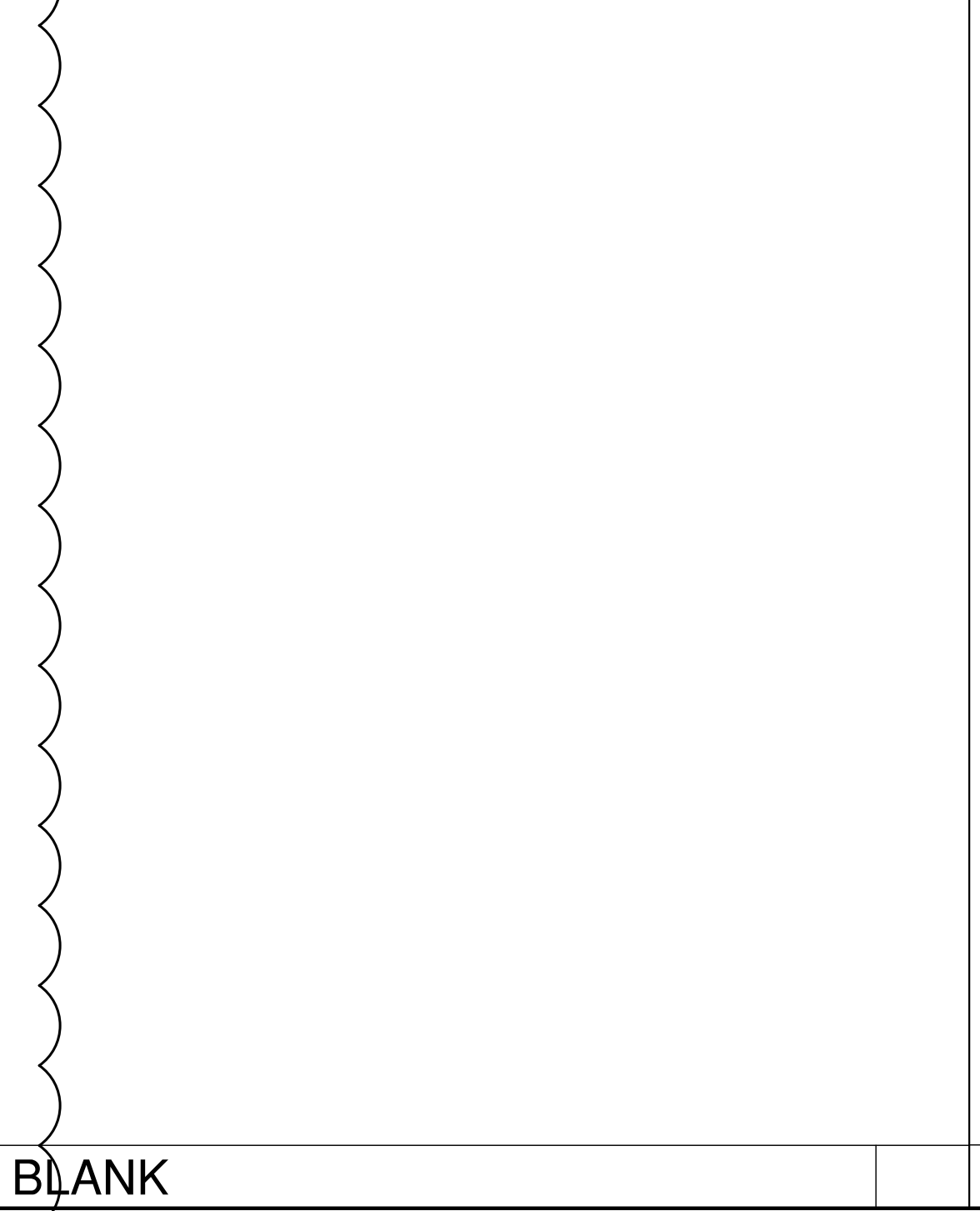
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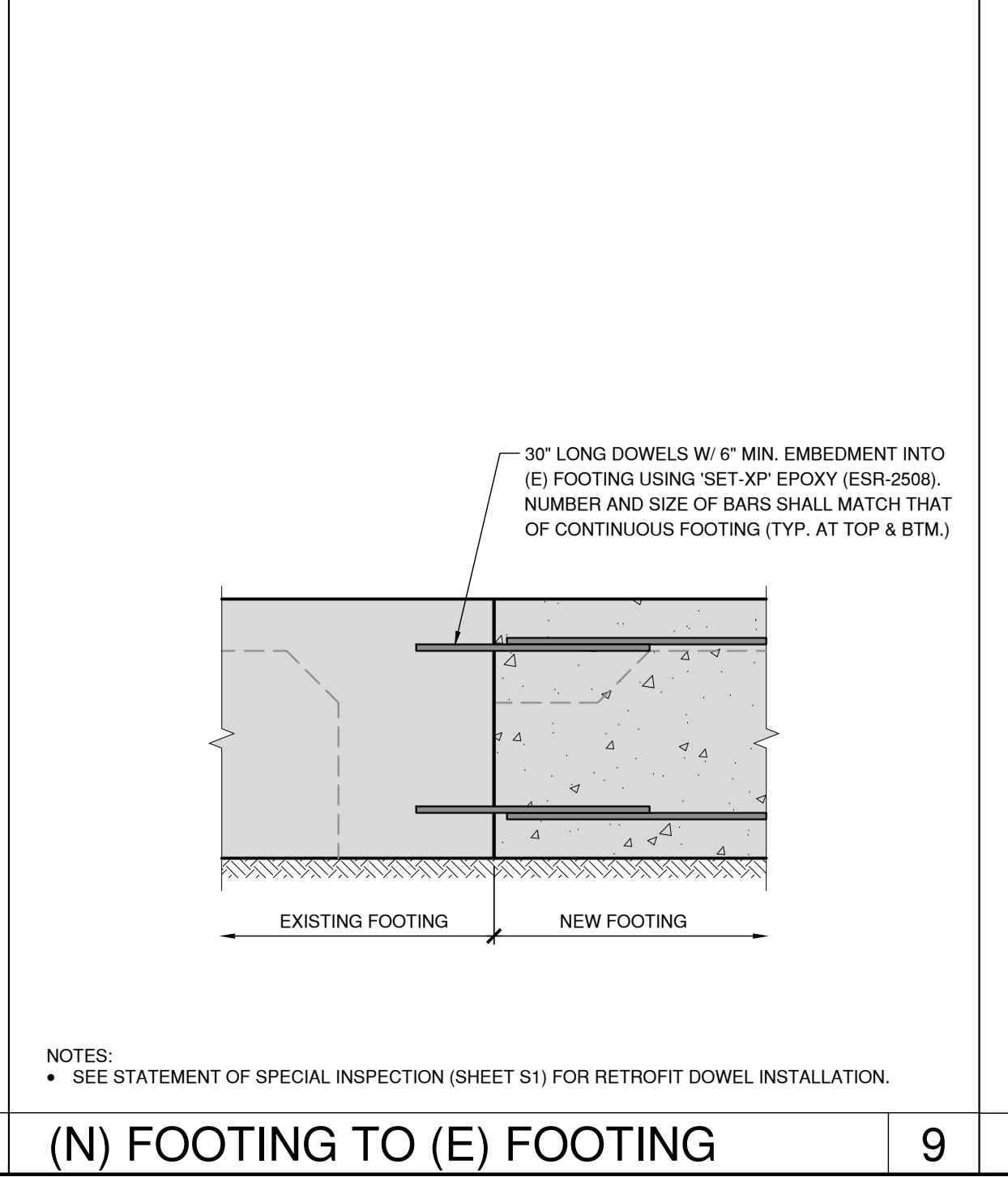
NOTES:
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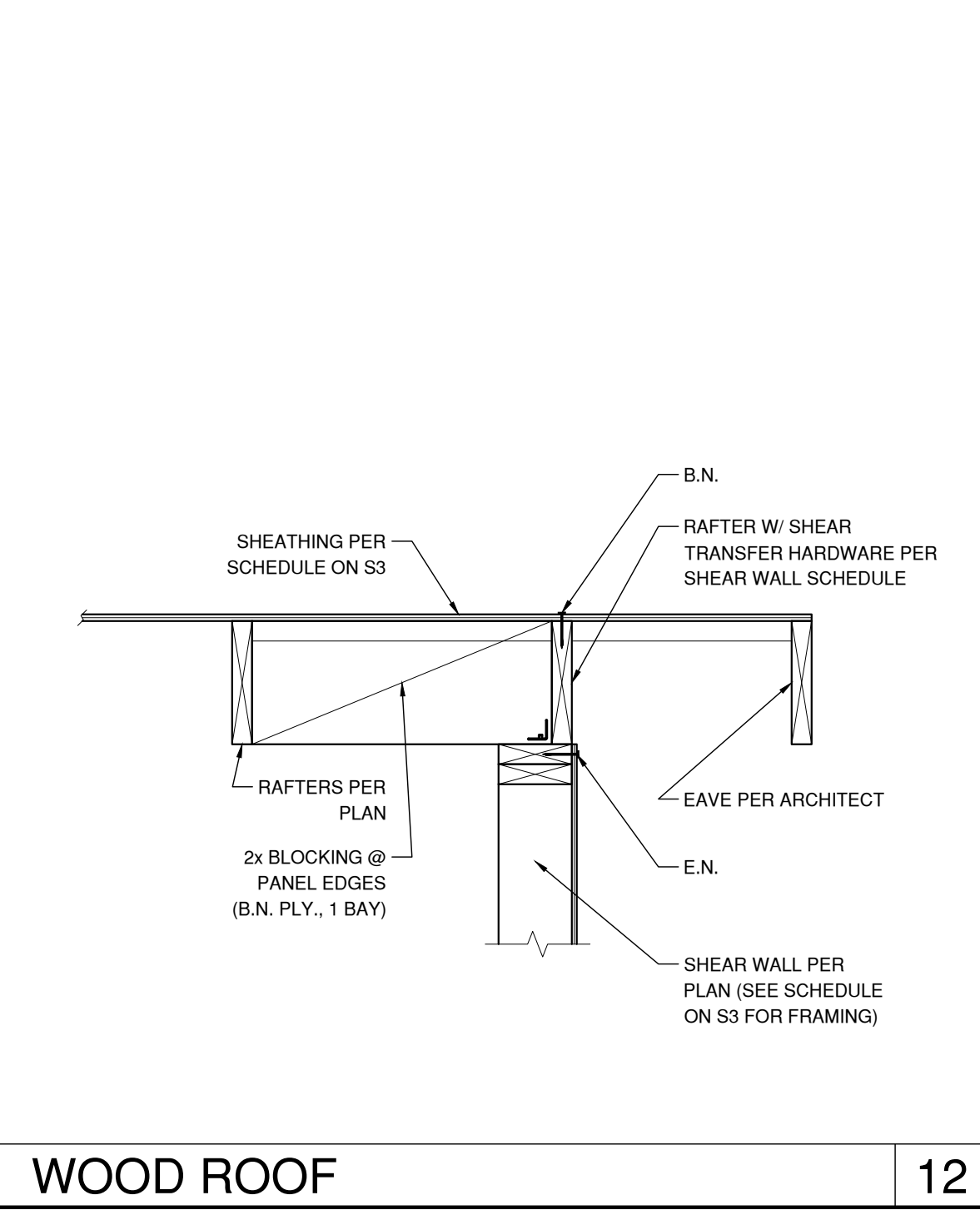
NOTES:
• RETROFIT ANCHOR BOLTS MAY BE REQUIRED IF EXISTING SILL PLATE IS CUT. SEE FOUNDATION NOTES FOR MAXIMUM DISTANCE OF ANCHOR BOLTS FROM END OF SILL PLATE.
• SEE STATEMENT OF SPECIAL INSPECTION (SHEET S1) FOR RETROFIT ANCHOR BOLT INSTALLATION.



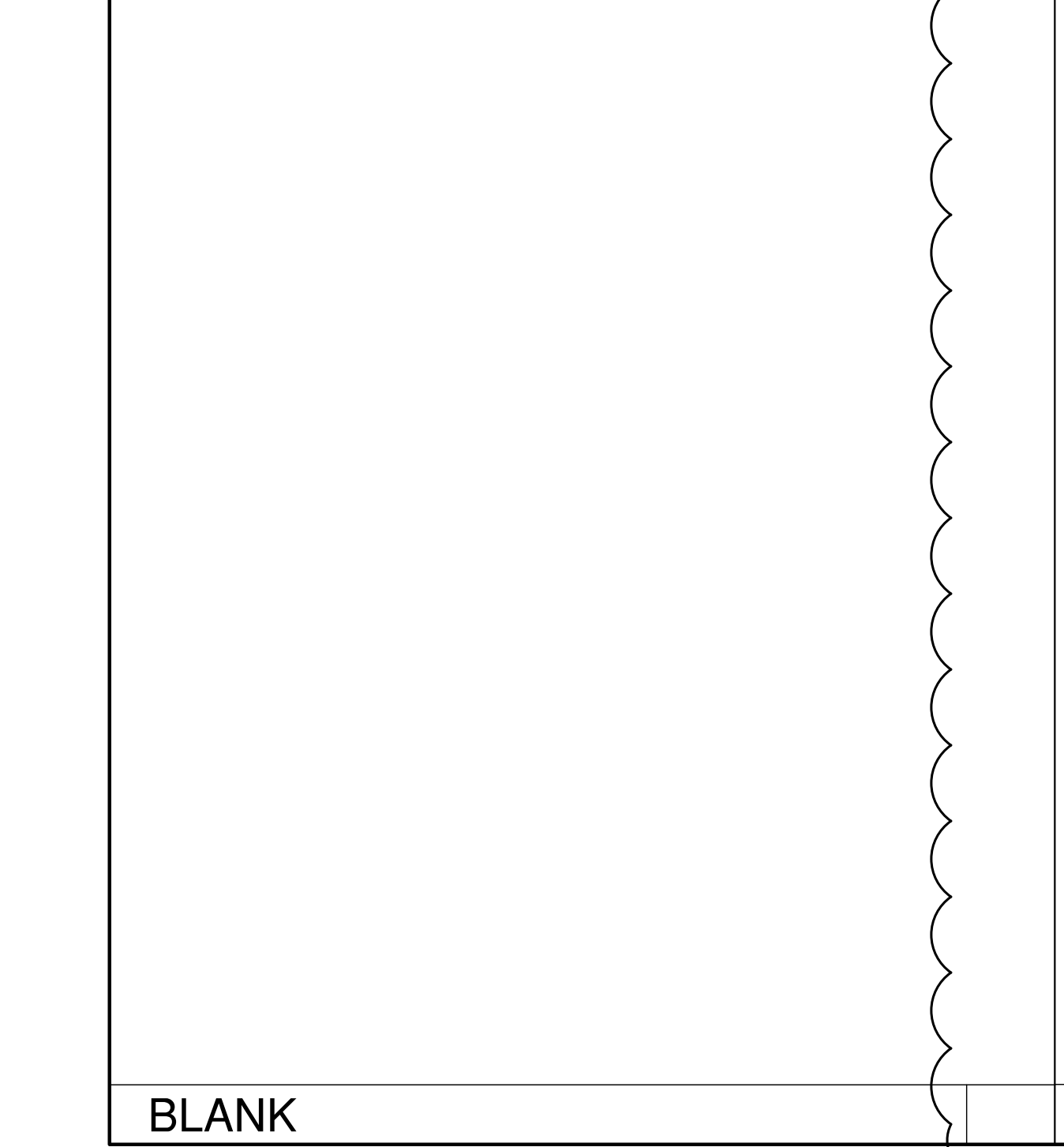
NOTES:
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